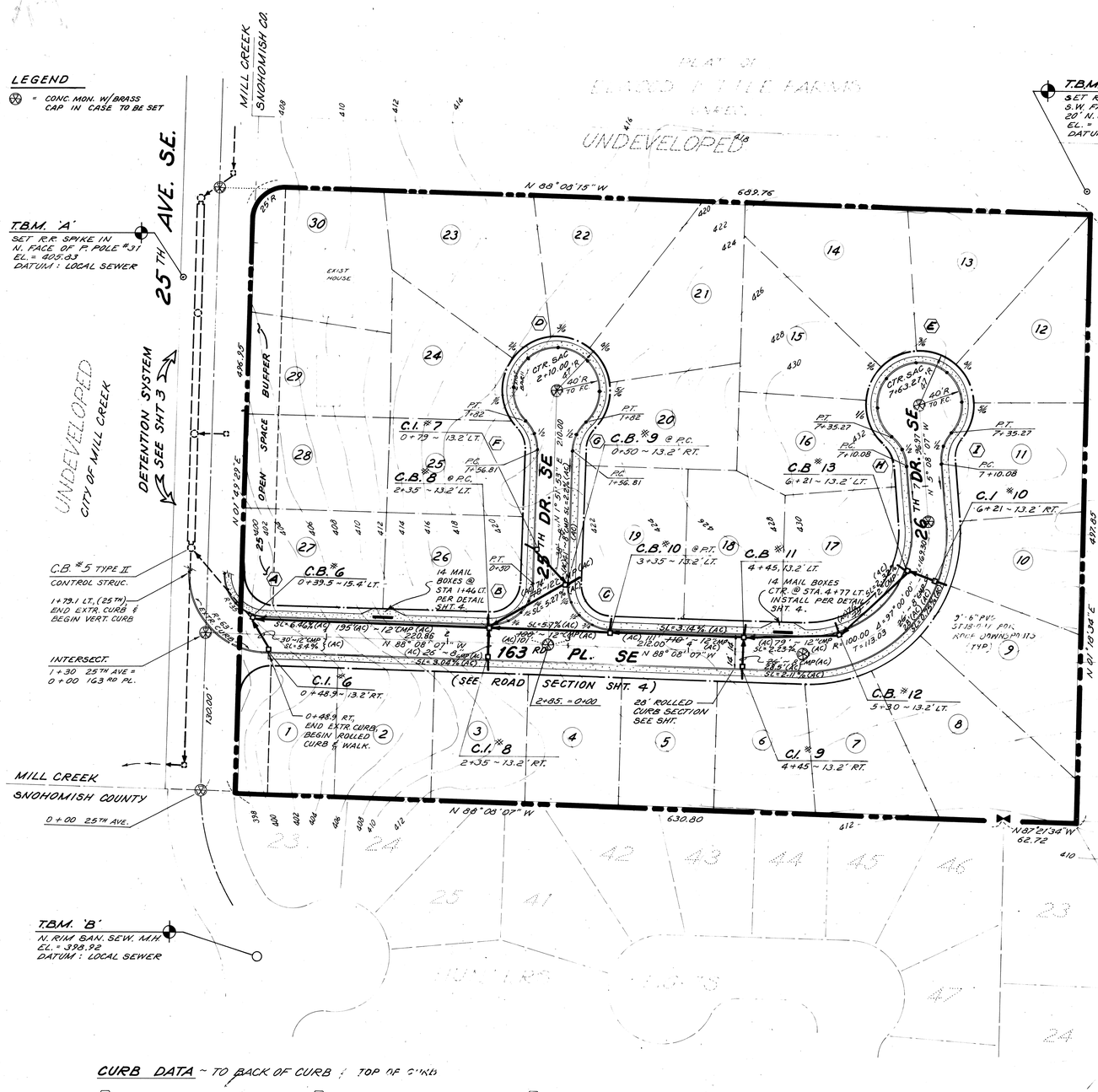
84-005
RD & STM. PLN.
MILL CREEK S.

S.W. 1/4, SEC. 3, TWP. 27, R.5 E., W.M.

LEGEND

⊗ = CONC. MON. W/BRASS CAP IN CASE TO BE SET



SCALE: 1" = 50'

GENERAL NOTES

1. ALL WORK AND MATERIALS TO BE IN ACCORDANCE WITH SNOHOMISH COUNTY STANDARDS AND SPECIFICATIONS.
2. PRIOR TO ANY SITE WORK PERTAINING TO DRAINAGE, THE CONTRACTOR SHALL CONTACT THE CHIEF INSPECTOR FOR LAND DEVELOPMENT DIVISION AT (206) 259-9444 TO SET UP SCHEDULING. DUE TO FIELD CHANGES (REVISIONS), ENGINEERED AS-BUILTS WILL BE REQUIRED PRIOR TO SITE APPROVAL.
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5. GRADING OUTSIDE OF THE COUNTY RIGHT-OF-WAY MUST COMPLY WITH CHAPTER 70 OF THE UNIFORM BUILDING CODE.
6. STORM SEWER PIPE MAY BE:
   - A. PLAIN CONCRETE PER AASHTO DESIGNATION M 86, CLASS 2. (RUBBER GASKET)
   - B. CORRUGATED STEEL PIPE, 16 GAUGE, GALVANIZED W/ASPHALT TREATMENT #1 OR BETTER. (RUBBER GASKET)
   - C. CORRUGATED ALUMINUM PIPE, 16 GAUGE IN R/W. (RUBBER GASKET)
   - D. ALUMINIZED STEEL MEETING AASHTO DESIGNATION M 36, 16 GAUGE. (RUBBER GASKET)
   - E. POLYVINYL CHLORIDE (P.V.C.) DRAIN PIPE MEETING AASHTO DESIGNATION M 278, 8" DIAMETER MAXIMUM. P.V.C. NOT ALLOWED IN THE R/W. (RUBBER GASKET)

   ALL PIPE SHALL HAVE CLASS "B" BEDDING UNLESS OTHERWISE SHOWN.
7. DETENTION PIPE SHALL BE 14 GAUGE CORRUGATED STEEL PIPE 35"x 73" CMPA WITH 3"x1" CORRUGATIONS AND ASPHALT TREATMENT NO. 1. CONNECTING BANDS SHALL BE MINIMUM 18" WIDE, WITH RUBBER GASKETS. COMPACTION TESTS WILL BE REQUIRED AROUND THE PIPE. NATIVE MATERIAL WILL NOT BE ALLOWED FOR BACKFILL AROUND PIPE. 100% IMPORTED GRAVEL BASE CLASS B WILL ONLY BE ACCEPTABLE. CONTRACTOR WILL SUPPLY METHOD OF COMPACTION AND LIST EQUIPMENT PRIOR TO WORK STARTING. COMPACTION TESTS BY MODIFIED PROCTOR METHOD.
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Gage	Pipe	Arch	Band Size
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10. THE BACKFILL SHALL BE PLACED EQUALLY ON BOTH SIDES OF THE PIPE OR PIPE-ARCH IN LAYERS WITH A LOOSE AVERAGE DEPTH OF 6", MAXIMUM DEPTH 8", THOROUGHLY TAMPING EACH LAYER. THESE COMPACTED LAYERS MUST EXTEND FOR ONE DIAMETER OR EACH SIDE OF THE PIPE OR TO THE SIDE OF THE TRENCH. MATERIALS TO COMPLETE THE FILL OVER PIPE SHALL BE THE SAME AS DESCRIBED. (REFER TO WSDOT STD. SPEC. 7-04.3(1) AND STD. SPEC. 2-03.3(14)C, METHOD B & C),
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2	~	PLAT, ROAD & STORM PROFILES
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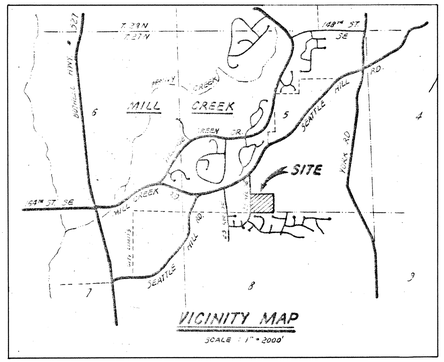
4	~	DETAILS
5	~	TEMP. EROSION-SEDIMENTATION CONTROL PLAN



CURB DATA ~ TO BACK OF CURB ; TOP OF CURB

(A) Δ = 90°07'24" R = 35' L = 50.05
P.C. = 0+48.9 = 400.00 (163 RD)
¼Δ = 400.37
½Δ = 400.74
¾Δ = 401.10
P.T. = 1+78.9 = 401.47 (25TH AVE.)

(B) Δ = 90°00'00" R = 36' L = 56.50
P.C. = 2+35 = 412.13 (163 RD)
¼Δ = 413.03
½Δ = 413.94
¾Δ = 414.84
P.T. = 0+50 = 415.74 (25TH DR.)

(C) Δ = 90°00'00" R = 36' L = 56.50'
P.C. = 3+50 = 415.74 (25TH DR.)
¼Δ = 416.38
½Δ = 417.01
¾Δ = 417.65
P.T. = 3+35 = 418.28 (163 RD)

(D) Δ = 268°49'50" R = 40' L = 187.68
¼Δ = 44°48'18"
P.C. = P.T. 1+32 = 418.20
⅛Δ = 418.51
⅜Δ = 418.90
⅝Δ = 419.70
⅞Δ = 419.39
⅛Δ = 419.00
P.T. = P.C. = 1+85 = 418.20

(E) Δ = 268°49'50" R = 40' L = 187.68
¼Δ = 44°48'18"
P.C. = P.T. = 7+35.27 = 426.69
¼Δ = 427.00
¾Δ = 427.45
⅜Δ = 428.18
⅝Δ = 427.45
⅞Δ = 427.00
PT = P.C. = 7+35.27 = 426.69

(F) Δ = 44°24'55" R = 36' L = 27.91'
P.C. = 1+56.81 = 417.80
½Δ = 418.00
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P.C. = 7+10.08 = 426.35
½Δ = 426.52
P.T. = 7+35.27 = 426.69

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CONSTRUCTION SCHEDULE

1) INSTALL T.E.S.C.P. (SEE SHT. 5)
2) CLEARING & GRADING (SEE SHT. 5)
3) SEWER AND WATER MAIN (A.W.D. PLANS)
4) STORM DRAINAGE FACILITIES
5) CURBS AND SIDEWALKS
6) PAVE STREETS

A.S.B.G.T.S. 3/1c/87

WE HEREBY CERTIFY THAT THE IMPROVEMENTS ARE LOCATED AS SHOWN ON THESE AS-BUILT PLANS.

PROJECT MANAGER OR SURVEYOR

DEVELOPER

REV. 1 12-11-86 RED
REV. 2 3-6-87 RED

APPROVED FOR CONSTRUCTION: Randolph A. Sleight for G.E. WEED, DIRECTOR, SNO. CO. DEPT. OF PUBLIC WORKS — DATE: 4/23/87

MAILBOX LOCATIONS APPROVED BY: MICHAEL MAHER, SUPERINTENDENT POSTAL OPERATIONS, BOTHELL — DATE: FEB. 27, 1987

NOTE: SEE SHEETS 2 & 3 FOR RD. & STORM AS-CONSTRUCTED DATA.

"AS - BUILT"

NO	REVISION	DATE	BY
REV. 4:	INCREASE C.D.S.'s R/W'S & FACE OF CURB RADII TO 47'R & 40'R RESPECTIVELY.	4-21-87	ALT
REV. 3:	REV. STORM SYSTEM 25TH & 163RD	4-10-87	A.J.
REV. 2:	ADD REV. GEN. NOTES, WALKS AROUND C.D.S.'s, REV. CURB DATA, ADD CONST. SCHED., RE ⊗ C.B.'s, & C.I.'s, AND ADD C.I. #9 & C.B. #11	3-06-87	A.J.
REV. 1:	ADDED STUB OUT FOR ROOF DRAINS, DELETED C.I. 9 & DR. ESM'T BETW. LOT 1 & 2.	12-11-86	D.S.

ROAD & STORM PLAN

FOR:

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

Western Surveyors Inc.
745-1594 or 355-2776
LAND USE CONSULTANTS
CIVIL ENGINEERS • LAND SURVEYORS
13322 Highway 99 South • Everett, WA 98204 • (206) 745-1594

DWN BY	DATE	PROJECT MANAGER	SCALE
KAM	8-26-86	EV. BONE	
CHKD BY	DATE	SHEET	JOB NO
EVB		1 OF 5	84-005

MILL CREEK SOUTH FIELD BOOK: 115

84-005

HDEV-284

84-005
RD & STM. PLN.
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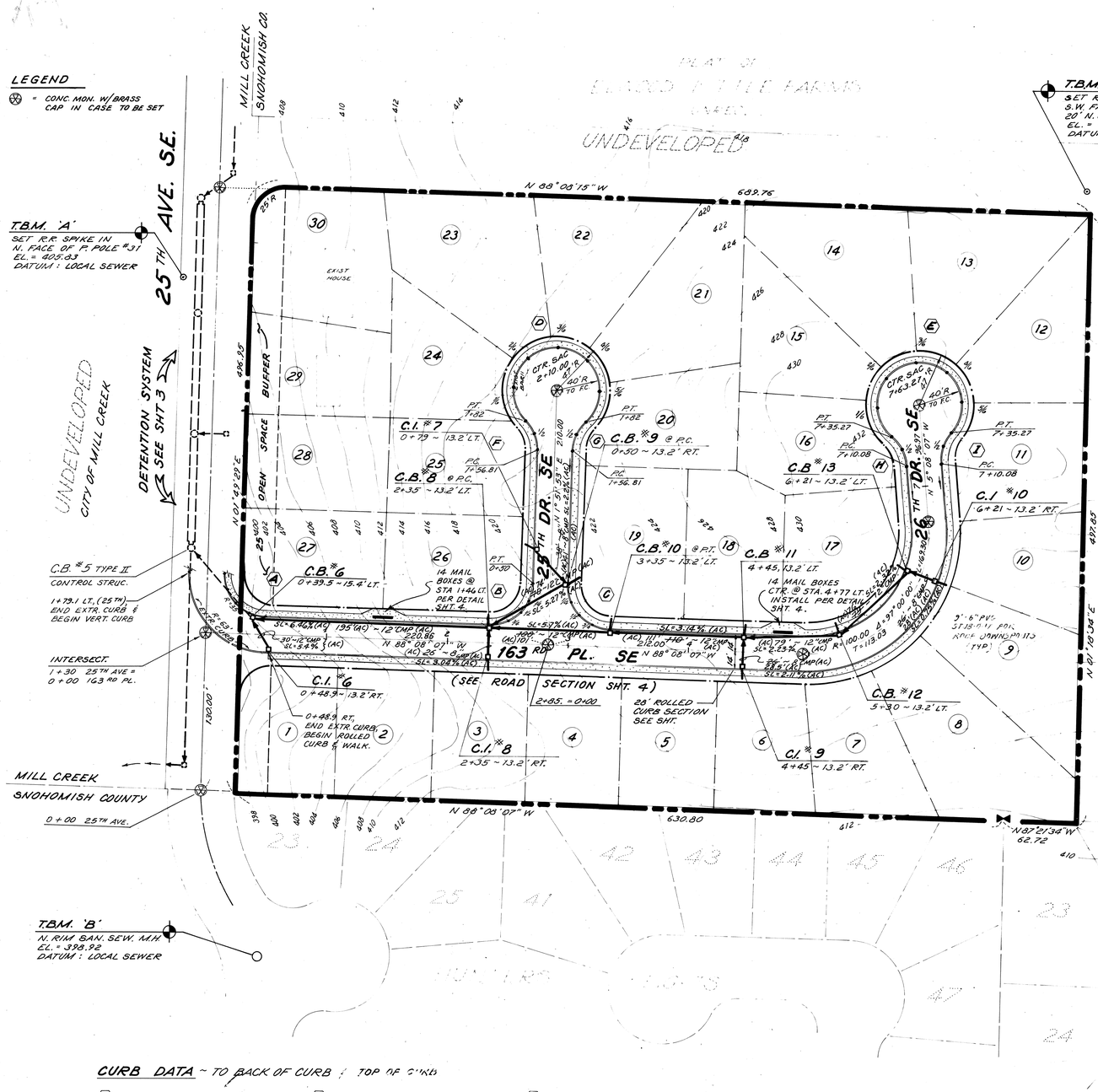

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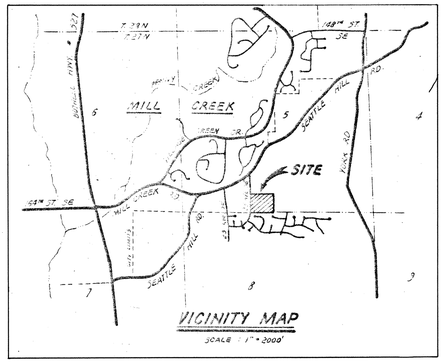

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½Δ = 413.94
¾Δ = 414.84
P.T. = 0+50 = 415.74 (25TH DR.)

(C) Δ = 90°00'00" R = 36' L = 56.50'
P.C. = 3+50 = 415.74 (25TH DR.)
¼Δ = 416.38
½Δ = 417.01
¾Δ = 417.65
P.T. = 3+35 = 418.28 (163 RD)

(D) Δ = 268°49'50" R = 40' L = 187.68
¼Δ = 44°48'18"
P.C. = P.T. 1+32 = 418.20
⅛Δ = 418.51
⅜Δ = 418.90
⅝Δ = 419.70
⅞Δ = 419.39
⅛Δ = 419.00
P.T. = P.C. = 1+85 = 418.20

(E) Δ = 268°49'50" R = 40' L = 187.68
¼Δ = 44°48'18"
P.C. = P.T. = 7+35.27 = 426.69
¼Δ = 427.00
¾Δ = 427.45
⅜Δ = 428.18
⅝Δ = 427.45
⅞Δ = 427.00
PT = P.C. = 7+35.27 = 426.69

(F) Δ = 44°24'55" R = 36' L = 27.91'
P.C. = 1+56.81 = 417.80
½Δ = 418.00
P.T. = 1+82 = 418.20

(G) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 1+56.81 = 417.80
½Δ = 418.00
P.T. = 1+82 = 418.20

(H) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 7+10.08 = 426.35
½Δ = 426.52
P.T. = 7+35.27 = 426.69

(I) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 7+10.08 = 426.35
½Δ = 426.52
P.T. = 7+35.27 = 426.69

CONSTRUCTION SCHEDULE

1) INSTALL T.E.S.C.P. (SEE SHT. 5)
2) CLEARING & GRADING (SEE SHT. 5)
3) SEWER AND WATER MAIN (A.W.D. PLANS)
4) STORM DRAINAGE FACILITIES
5) CURBS AND SIDEWALKS
6) PAVE STREETS

A.S.B.G.T.S. 3/1c/87

WE HEREBY CERTIFY THAT THE IMPROVEMENTS ARE LOCATED AS SHOWN ON THESE AS-BUILT PLANS.

PROJECT MANAGER OR SURVEYOR

DEVELOPER

REV. 1 12-11-86 RED
REV. 2 3-6-87 RED

APPROVED FOR CONSTRUCTION: Randolph A. Sleight for G.E. WEED, DIRECTOR, SNO. CO. DEPT. OF PUBLIC WORKS — DATE: 4/23/87

MAILBOX LOCATIONS APPROVED BY: MICHAEL MAHER, SUPERINTENDENT POSTAL OPERATIONS, BOTHELL — DATE: FEB. 27, 1987

NOTE: SEE SHEETS 2 & 3 FOR RD. & STORM AS-CONSTRUCTED DATA.

"AS - BUILT"

NO	REVISION	DATE	BY
REV. 4:	INCREASE C.D.S.'s R/W'S & FACE OF CURB RADII TO 47'R & 40'R RESPECTIVELY.	4-21-87	ALT
REV. 3:	REV. STORM SYSTEM 25TH & 163RD	4-10-87	A.J.
REV. 2:	ADD REV. GEN. NOTES, WALKS AROUND C.D.S.'s, REV. CURB DATA, ADD CONST. SCHED., RE ⊗ C.B.'s, & C.I.'s, AND ADD C.I. #9 & C.B. #11	3-06-87	A.J.
REV. 1:	ADDED STUB OUT FOR ROOF DRAINS, DELETED C.I. 9 & DR. ESM'T BETW. LOT 1 & 2.	12-11-86	D.S.

ROAD & STORM PLAN

FOR:

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

Western Surveyors Inc.
745-1594 or 355-2776
LAND USE CONSULTANTS
CIVIL ENGINEERS • LAND SURVEYORS
13322 Highway 99 South • Everett, WA 98204 • (206) 745-1594

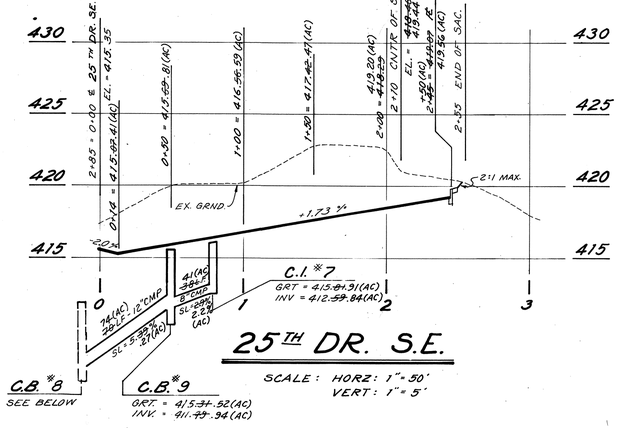
DWN BY	DATE	PROJECT MANAGER	SCALE
KAM	8-26-86	EV. BONE	
CHKD BY	DATE	SHEET	JOB NO
EVB		1 OF 5	84-005

MILL CREEK SOUTH FIELD BOOK: 115

84-005

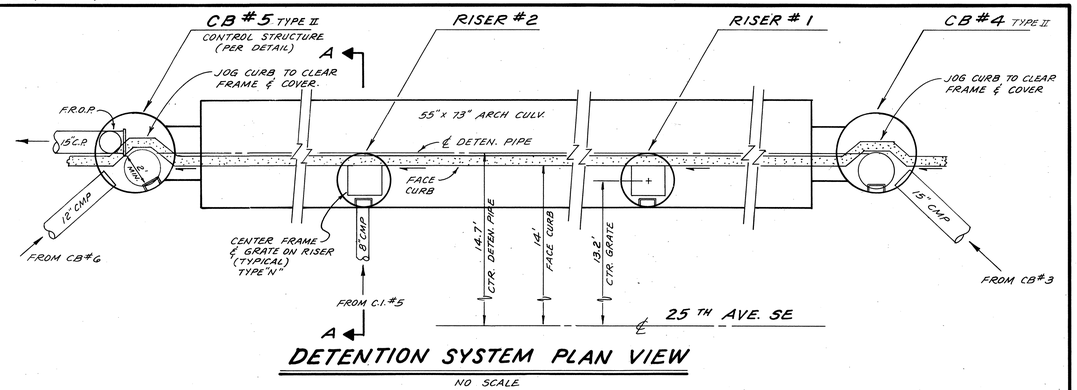
HDEV-284

84-005 2/5
RD & STM. PROF.
MILL CREEK S.



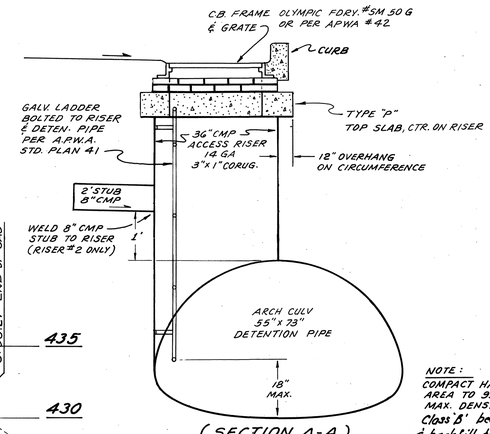
25TH DR. S.E.

SCALE: HORZ: 1"=50'
VERT: 1"=5'



DETENTION SYSTEM PLAN VIEW

NO SCALE

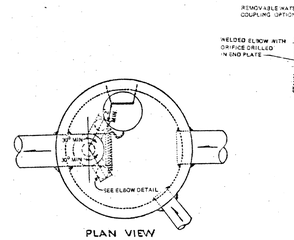


(SECTION A-A)

ACCESS RISER DETAIL

NO SCALE

NOTE:
COMPACT HAUNCH AREA TO 95% MAX. DENSITY. Class 'B' bedding & backfill to grade

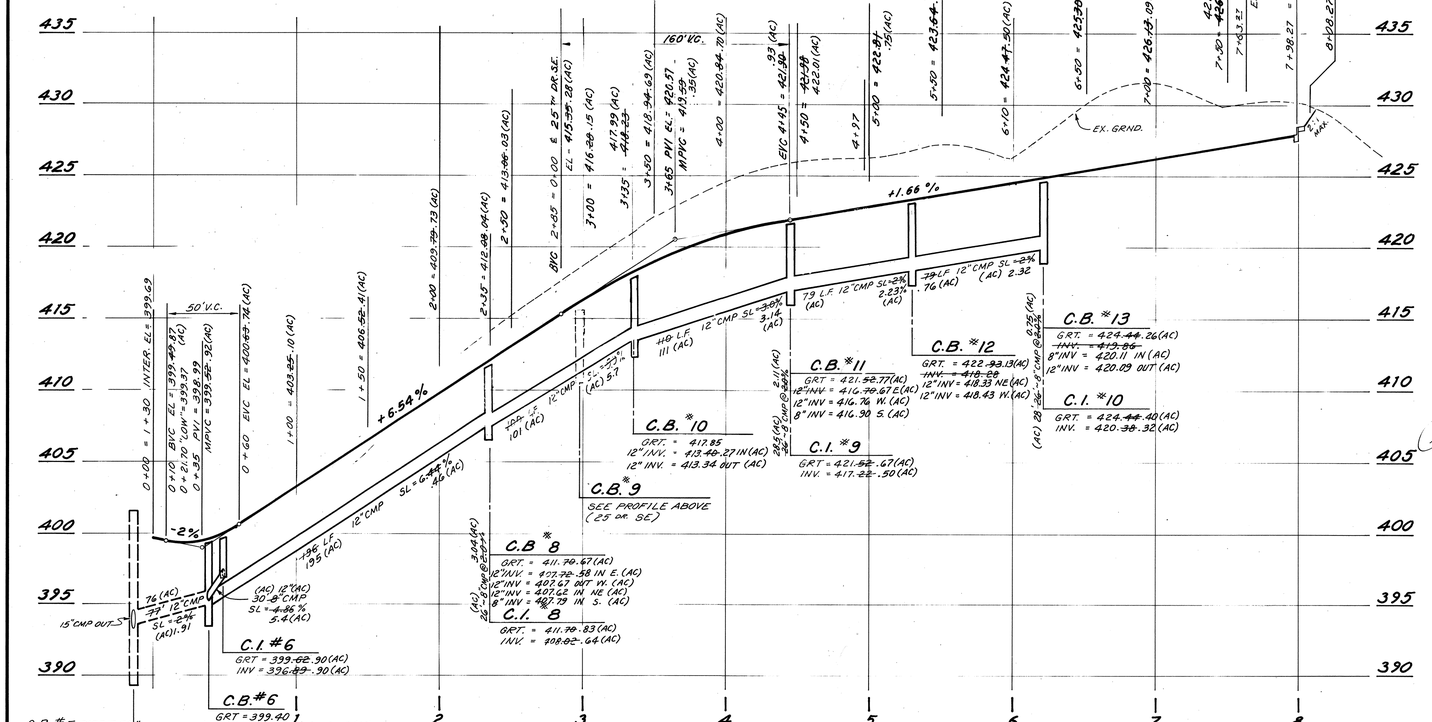


NOTES:
1. PIPE SIZES AND SLOPES: PER PLANS.
2. OUTLET CAPACITY: NOT LESS THAN COMBINED INLETS.
3. METAL PARTS: CORROSION RESISTANT. GALVANIZED PIPE PARTS TO HAVE ASPHALT TREATMENT.
4. FRAME & LADDER OR STEPS OFFSET SO A. CLEANOUT GATE IS VISIBLE FROM TOP.
3. CLIMB-DOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE.
C. FRAME IS CLEAR OF CURB.
5. IF METAL OUTLET PIPE CONNECTS TO CONCRETE CONCRETE PIPE, OUTLET PIPE TO HAVE SMOOTH O.D. EQUAL TO CONCRETE PIPE I.D. LESS 1/4".
6. MULTI-ORIFICE ELBOW MAY BE LOCATED AS SHOWN OR ENTIRELY ON ONE SIDE OF RISER ALLOWING AGGREGATE LADDER ACCESS.

MULT-ORIFICE F.R.O.P. CONTROL DEVICE

C.B. #~~4~~ #5 (AC) DETAIL

NO SCALE



163RD PL. S.E. & 26TH DR. S.E.

SCALE: HORZ: 1"=50'
VERT: 1"=5'

WE HEREBY CERTIFY THAT THE IMPROVEMENTS ARE LOCATED AS SHOWN ON THESE AS-BUILT PLANS.

PROJECT MANAGER OR SURVEYOR

DEVELOPER

NOTE:
(AC) REPRESENTS AS CONSTRUCTED DATA

AS-BLTS 8/10/87

APPROVED FOR CONSTRUCTION BY: G.E. WEED, DIRECTOR DEPT. OF PUBLIC WORKS DATE: 4/23/87

"AS - BUILT"

REV. 2: REV. ROAD & STORM PROFILES @ INTERSECTION 25TH AVE. & 163 RD PL. 4-10-87 A.J.
REV. 1: REV. ROAD & STORM DR. PROFILE. 12-11-86 D.S.

RICHARD E. SCHMIDT 10-21-86 PROFESSIONAL ENGINEER

REV. 1 12-11-86 RES
REV. 2 3-6-87 RES

ROAD & STORM PROFILES

FOR:

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

NO.	REVISION	DATE	BY

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13322 Highway 99 South • Everett, WA 98204 • (206) 745-1594

DWN. BY	DATE	PROJECT MANAGER	SCALE
KAM	8-29-86	E.J. BONE	AS SHOWN
CHKD. BY	DATE	SHEET	JOB NO.
EJB		2 OF 5	84-005

HOEV-285

84-005 3/5
RD; STM. PLAN
MILL CREEK STH.

S.W. 1/4, SEC. 5, T.WP. 27, R.5 E, W.M.

"SEE DETENTION SYSTEM DETAILS SHT. 2"

C.B. #5 TYPE II 54"
2+00 ~ 14.67' LT.
CTR. M.H. W/SLOTTED OPEN COVER
O.F. # MH 40G

RISER #2 ACCESS RISER 36"
W/TYPE "N" TOP SLAB & LOCKING OPEN GRATE
STA 2+95 LT.
2.178 (AC)

RISER #1 ACCESS RISER 36"
W/TYPE "N" TOP SLAB & LOCKING OPEN GRATE
STA ~~3+79~~ LT.
3+88 (AC)

STA 4+87 (AC)
CB #4 TYPE II 54"
~~4+90~~ ~ 14.67' LT. CTR MH
W/SLOTTED OPEN COVER
O.F. # MH 40G

SCALE: 1" = 50'

T.B.M. "A"
SET R.R. SPIKE IN N. FACE OF P. POLE #31
ELEV. = 405.83
DATUM: LOCAL SEWER

EXIST. EMERGENCY OVERFLOW
WET AREA
EXIST. DETENTION POND
EXISTING DRAINAGE ROUTE THRU WOODS
C.B. #7
0+20 ~ 16 LT.
STUB TO R/W W/15" C.P.
RAISE SEWER M.H. TO FINISH GRADE
0+00
1+30 = 0+00 (AC)
180 LF ~ 15" C.P. (AC)
SL = ~~0.5~~ 0.6 % (AC)
130.00

(SEE ROAD SECTION SHT. 4)
25TH AVE. SE.

N 01°44'29" E
278.5 (AC)
N 01°44'29" E
N 01°44'29" E

RIP-RAP, 4' ROCK FOR EMER. SPILLWAY (AC) TO STA 0+30 (25TH); DIRECT TO W. R/W & END
1
C.I. #6
(SEE SHT. 2)

C.B. #6
(SEE SHT. 2)
EMERGENCY OVERFLOW
GRT = ~~339.46~~ 338.88 (AC)
LEAVE 1 GAP IN EXTR. CURB @ STA 0+21 RT. (163RD PL.)

(AC) DOUBLE ACCESS CURB CUT CTR. @ 3+09.9 20' OPENING
C.I. #5
2+95 ~ 13.2' RT.

(AC) SINGLE ACCESS CURB CUT CTR. @ 4+60 20' OPENING

C.B. #3
5+07 13.2' RT. (AC)

C.B. #2
~~3+82~~ ~ 13.2' RT.
5+83 (AC)

C.I. #3
7+00 ~ 13.2' LT.

28' - VERTICAL CURB & GUTTER SECTION. SEE DETAIL SHT.

C.B. #1
7+82 ~ 12' RT.
ADD LOCK'G LID & MATCH SIDEWALK

C.I. #4
~~7+00~~ ~ 13.2' RT.
7+01.5 (AC)

C.I. #1
8+77 ~ 13.2' LT. (AC)

8+86
MATCH TO EXIST. ROAD

(EXIST.) 125'-15" CMP. TO BE REMOVED

(EXIST.) 177'-15" CMP. SL = 0.32 % (TO REMAIN)

C.I. #2
8+77 ~ 13.2' RT.

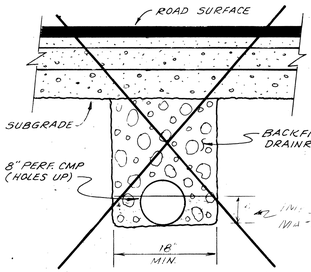
(EXIST.) C.B.
8+77 ~ 13.5' RT.
ADD LOCK'G LID & RAISE, MATCH SIDEWALK

SEATTLE HILL ROAD
160TH PL. SE.
163RD PL. SE.

HUNTER HEIGHTS
MILL CREEK SOUTH
ELWOOD LITTLE FARMS (UNREC.)
MILL CREEK ESTATES

WE HEREBY CERTIFY THAT THE IMPROVEMENTS ARE LOCATED AS SHOWN ON THESE AS-BUILT PLANS.
PROJECT MANAGER OR SURVEYOR
DEVELOPER

AS-BUILTS 8/10/87



~~TYPICAL INLET CONNECTORS~~ (25TH AVE. SE)
~~SECTION A-A~~ (NOT USED) (AC)
NO SCALE

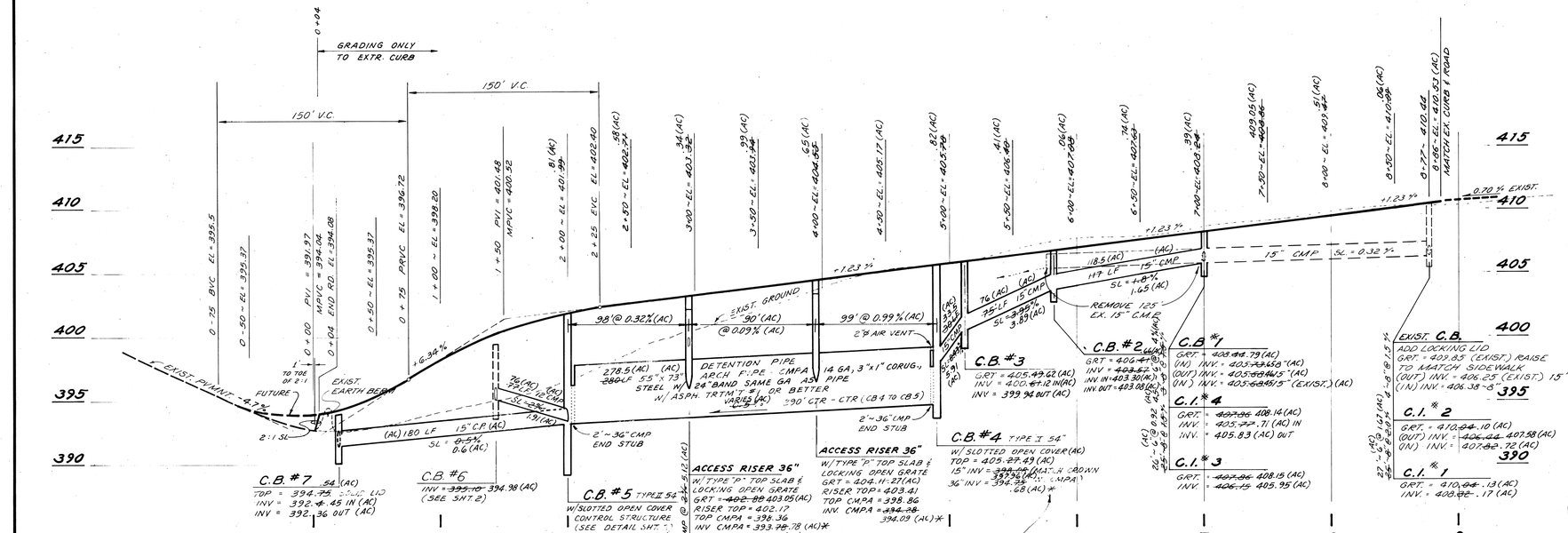
25TH AVE. SE APPROVED FOR CONSTRUCTION: J. Allan Newhill, CITY OF MILL CREEK — DATE: 4-23-87

APPROVED FOR CONSTRUCTION: Ronald R. Wright BY: G.E. WEED, DIRECTOR, DEPT. OF PUBLIC WORKS — DATE: 4/23/87

NOTE: (AC) REPRESENTS AS CONSTRUCTED DATA.

"AS - BUILT"

REV. 3: MOVE DETENTION SYSTEM TO W. SIDE OF 25TH, REV. ROAD PROFILE SOUTH OF 2+25. 4-10-87 A.J.
REV. 2: REVISE EMER. OVERFLOW, ADD C.B.@ 5+82, ADD SEC. A-A 3-6-87 A.J.
REV. 1: REVISE ROAD PROFILE 12-11-86 D.S.



AS-BUILT DETENTION SYSTEM VOLUME = 6850 ft³ ±

REV. 1 12-11-86
REV. 2 3-6-87

ROAD AND STORM PLAN & PROFILE

FOR:

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

NO	REVISION	DATE	BY

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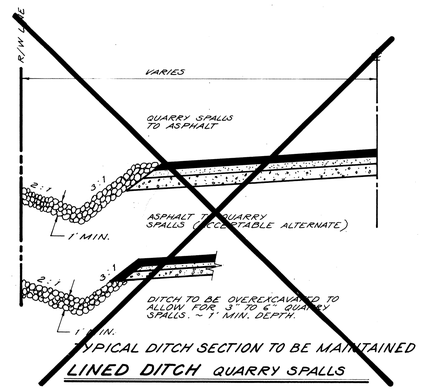
DWN BY	DATE	PROJECT MANAGER	SCALE
KAM	8-29-86	E.J. BONE	AS SHOWN
CHKD BY	DATE	SHEET	JOB NO.
EJB		3 OF 5	84-005

MILL CREEK SOUTH JOB NO. 84-005

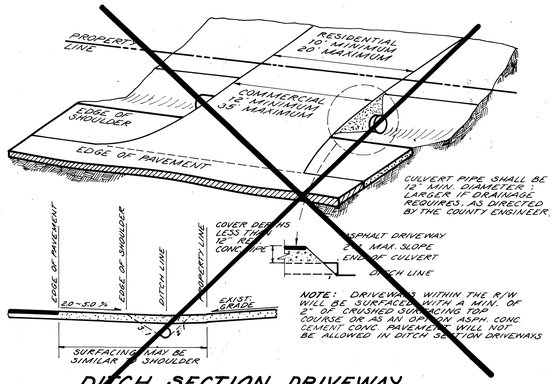
HDEV-286

84-005 4/5
DETAILS
MILL CREEK S.

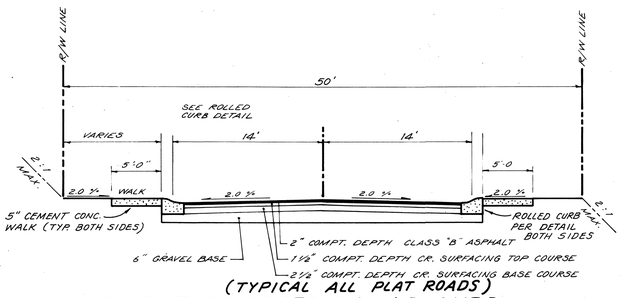


TYPICAL DITCH SECTION TO BE MAINTAINED

LINED DITCH QUARRY SPALLS

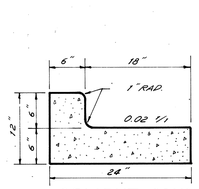


DITCH SECTION DRIVEWAY

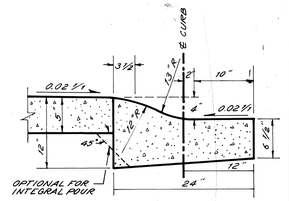


(TYPICAL ALL PLAT ROADS)

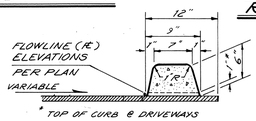
CEMENT CONCRETE ROLLED CURB
CURB ROADWAY SECTION



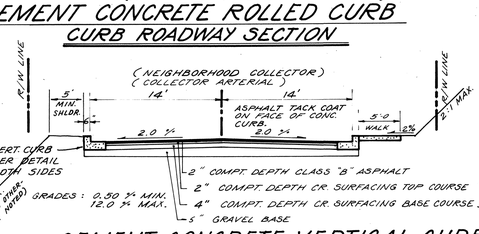
CEMENT CONC. VERTICAL CURB & GUTTER



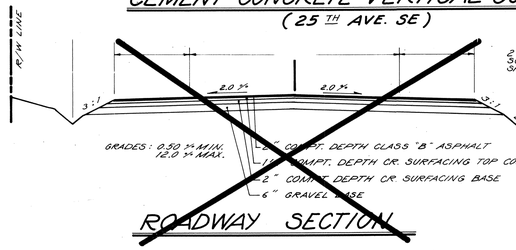
CEMENT CONC. ROLLED CURB



EXTRUDED ASPHALT OR CEMENT CONC. CURB

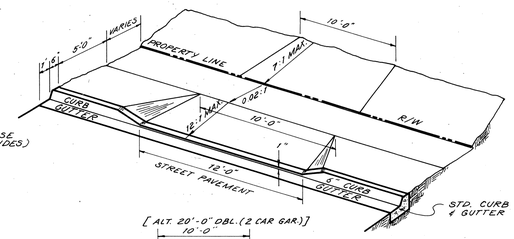


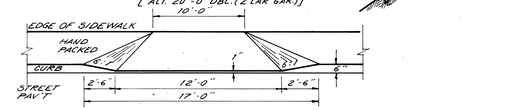
CEMENT CONCRETE VERTICAL CURB
(25TH AVE. SE)



ROADWAY SECTION

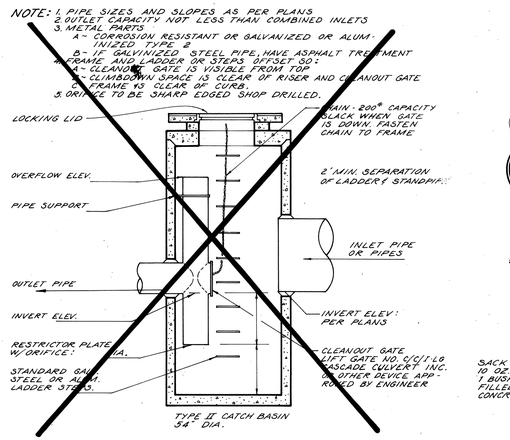
2" COMPT. DEPTH CR. SURFACING TOP COURSE, SHOULDERS (BOTH SIDES)



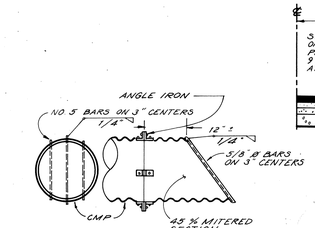


CEMENT CONC. OR ASPHALT CONC. DRIVEWAY DETAILS

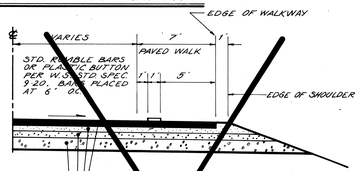
NOTE: 1. PIPE SIZES AND SLOPES AS PER PLANS.
2. OUTLET CAPACITY NOT LESS THAN COMBINED INLETS
3. METAL PARTS:
A - CORROSION RESISTANT OR GALVANIZED OR ALUMINIZED TYPE 2
B - IF GALVINIZED STEEL PIPE, HAVE ASPHALT TREATMENT
4. FRAME AND LADDER OR STEPS OFFSET 30°:
A - CLEANOUT GATE IS VISIBLE FROM TOP
B - CLIMBDOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE
C - FRAME 45° CLEAR OF CURB
5. ORIFICE TO BE SHARP EDGED SHOP DRILLED.



F.R.O.P. CONTROL DEVICE DETAIL



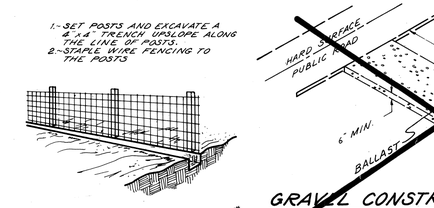
TRASH SCREEN DETAIL

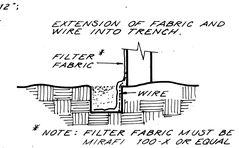


ROADWAY SECTION

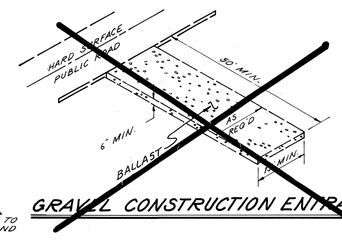
THE RUMBLE BARS SHALL BE A SOLID POLYESTER CASTING OF THE FOLLOWING DIMENSIONS: WIDTH ~ 4", LENGTH ~ 12", AND HEIGHT VARYING FROM 1" AT THE EDGE OF THE BAR TO 1/4" ALONG THE LONGITUDINAL CENTERLINE. THE ENDS OF THE BAR SHALL BE BEVELED FROM THE TOP OF THE 1/4" EDGE AT A 1:1 SLOPE.

STANDARD WALKWAY

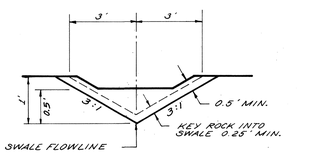




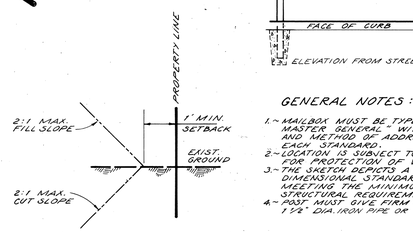
FILTER FENCE DETAIL



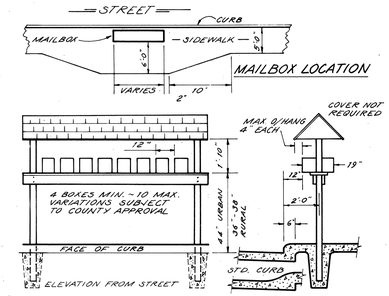
GRAVEL CONSTRUCTION ENTRANCE



INTERCEPTOR DITCH X-SECTION & ROCK CHECK DAM



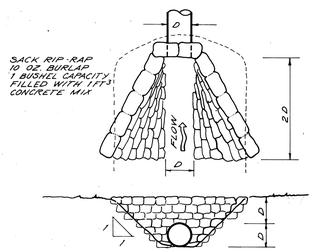
CUT & FILL REQUIREMENTS



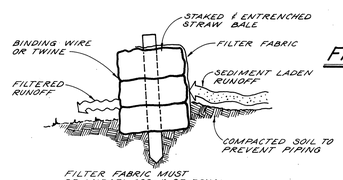
MAILBOX STANDARD

GENERAL NOTES:
1.- MAILBOX MUST BE TYPE "APPROVED BY THE POST-MASTER GENERAL" WITH A UNIFORM BOX STYLE AND METHOD OF ADDRESS IDENTIFICATION PER EACH STANDARD.
2.- LOCATION IS SUBJECT TO APPROVAL BY THE COUNTY FOR PROTECTION OF VIEWS AND ACCESS.
3.- THE METHOD DEPICTS A MINIMUM STRUCTURAL AND DIMENSIONAL STANDARD. INNOVATIVE DESIGNS MEETING THE MINIMUM DIMENSIONAL AND STRUCTURAL REQUIREMENTS ARE ACCEPTABLE.
4.- POST MUST GIVE FIRM SUPPORT BUT NOT TO EXCEED 1 1/4" DIA. IRON PIPE OR 4"x4" WOODEN POST.

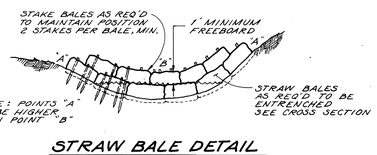
APPROVED FOR CONSTRUCTION — BY: G.E. WEED, DIRECTOR, DEPT. OF PUBLIC WORKS — DATE: 4/23/87



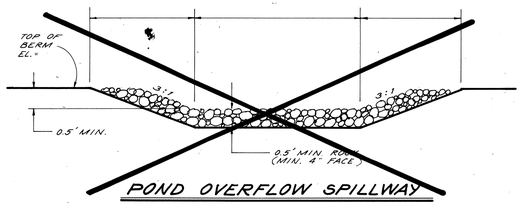
HEADWALL
SACK RIP-RAP DETAIL



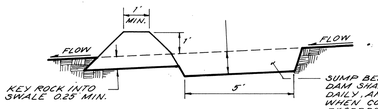
STRAW BALE CROSS-SECTION



STRAW BALE DETAIL



POND OVERFLOW SPILLWAY



NOTE: ROCK SHALL BE 4" MINUS QUARRY ROCK

ROCK CHECK DAM SECTION

TYPICAL INTERCEPTOR DITCH WITH ROCK CHECK DAM

R.E. Schmidt 10-22-86

Rev. 1 3-10-87 RES

Lee Hunt 4-10-87

REV. 1 : REV. RD. SEC. (25TH) 3/6/87 AJ.

NO	REVISION	DATE	BY

Western Surveyors Inc.
745-1594 or 355-2776

LAND USE CONSULTANTS
CIVIL ENGINEERS • LAND SURVEYORS
13322 Highway 99 South • Everett, WA 98204 • (206) 745 1594

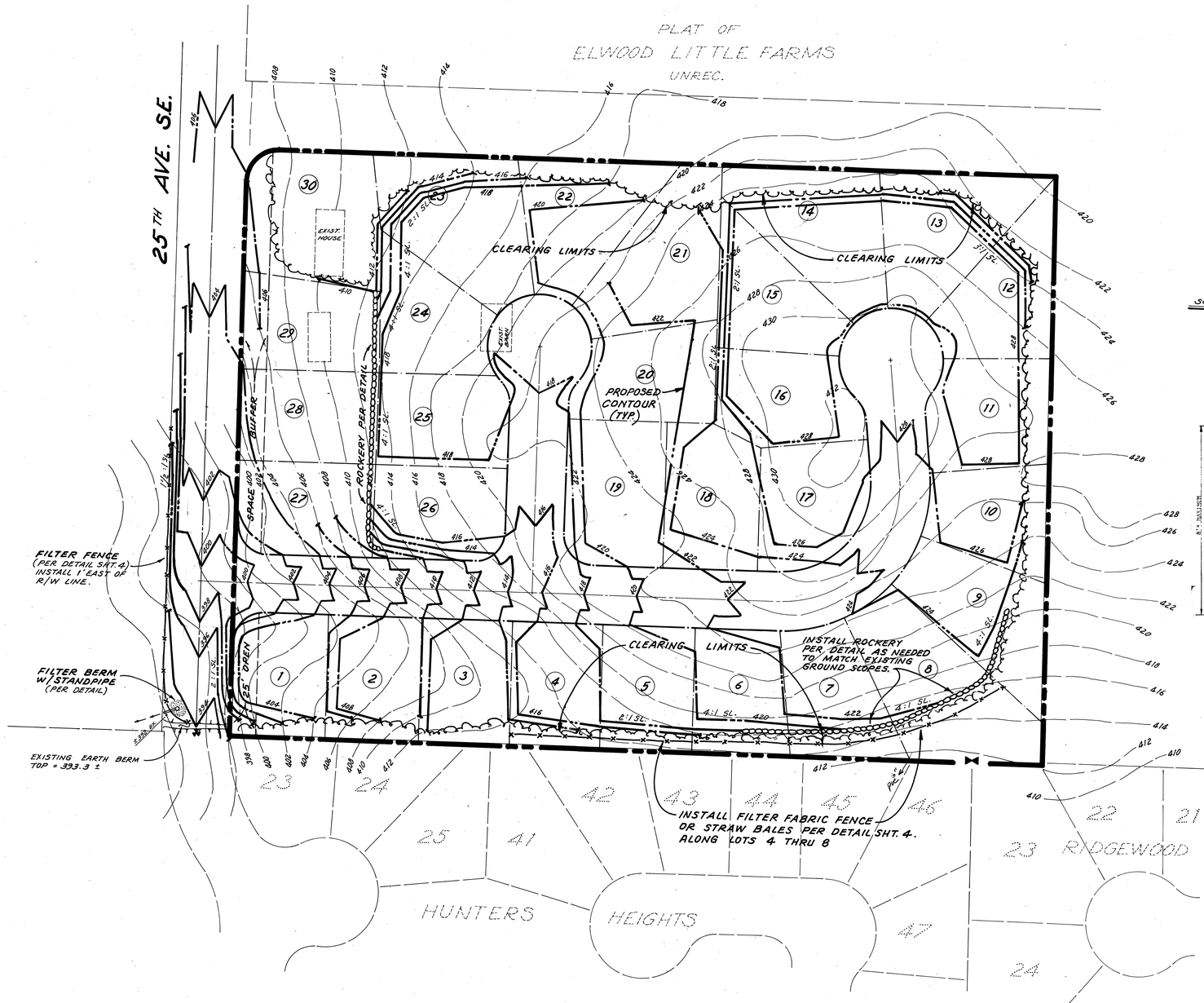
DWN BY: DATE	PROJECT MANAGER	SCALE
KAM 8-26-86	E.J. BONE	AS SHOWN
CHKD BY: DATE	SHEET	JOB NO
RES	4 OF 5	84-005

DETAILS
FOR:
MILL CREEK SOUTH
DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

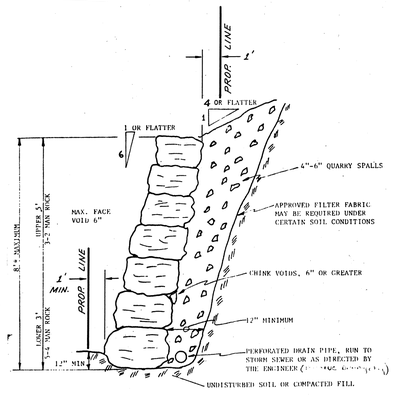
HDEV-287

MILL CREEK SOUTH 84-005

S.W. 1/4, SEC. 5, TWP. 27, R.5 E., W.M.



SCALE : 1" = 50'



THE LONG DIMENSION OF THE ROCKS SHALL EXTEND INTO THE EARTH TO PROVIDE MAXIMUM STABILITY

THE ROCK SHALL BE PLACED SO AS TO LOCK INTO TWO ROCKS IN THE TIER

2 MAN ROCKS	300 LBS. MIN.,	800 LBS. MAX.,	SIZE 13" TO 18"
3 MAN ROCKS	800 LBS. "	1500 LBS. MAX.,	SIZE 18" TO 24"
4 MAN ROCKS	1500 LBS. "	2100 LBS. MAX.,	SIZE 24" TO 32"
5 MAN ROCKS	2100 LBS. "	3000 LBS. MAX.,	SIZE 30" TO 38"

ANY ROCKERY GREATER THAN 8 FEET IN HEIGHT MUST BE DESIGNED AND INSPECTED BY A LICENSED STRUCTURAL ENGINEER. THE ENGINEER MUST CERTIFY TO THE CITY THAT THE ROCKERY WAS INSTALLED IN ACCORDANCE WITH PLANS AND SPECIFICATIONS.

ROCKERIES 7 FEET AND LOWER SHALL BE CONSTRUCTED OF 4 MAN TO 2 MAN ROCKS FROM BOTTOM TO TOP.

ROCKERIES 7 FEET AND HIGHER SHALL BE CONSTRUCTED OF 5 MAN TO 2 MAN ROCKS FROM BOTTOM TO TOP.

ROCKERY DETAIL

NO SCALE

TEMPORARY EROSION/SEDIMENTATION CONTROL NOTES

1. WHERE POSSIBLE, MAINTAIN NATURAL VEGETATION FOR SILT CONTROL.
2. TEMPORARY SILTATION AND DETENTION PONDS WILL BE REQUIRED. ONE METHOD OF CONSTRUCTION ACCEPTABLE TO THE COUNTY IS BY PLACING STRAW BALES ACROSS SWALES HELD BY TWO STAKES PER BALE. RIP-RAP SHALL BE USED ON BOTH SIDES OF BALES FOR EROSION CONTROL AS REQUIRED.
3. ALL TEMPORARY SILTATION AND DETENTION PONDS SHALL BE MAINTAINED IN A SATISFACTORY CONDITION UNTIL SUCH TIME THAT CLEARING AND OR CONSTRUCTION IS COMPLETED AND THE PERMANENT DRAINAGE FACILITIES ARE OPERATIONAL.
4. AFTER CONSTRUCTION, TEMPORARY SILTATION CONTROL AREAS SHOULD BE EITHER RETURNED TO ORIGINAL GROUND CONDITIONS OR PHASED INTO PERMANENT APPROVED DRAINAGE PLAN.
5. THE SILTATION CONTROL SYSTEMS DEPICTED ON THIS DRAWING ARE SUCH AS TO MEET MINIMUM COUNTY REQUIREMENTS. AS CONSTRUCTION PROGRESSES AND EXPECTED SEASONAL CONDITIONS DICTATE, MORE SILTATION CONTROL FACILITIES MAY BE REQUIRED TO INSURE COMPLETE SILTATION CONTROL OF THIS PROPERTY. THEREFORE, DURING THE COURSE OF CONSTRUCTION, IT SHALL BE THE OBLIGATION OF THE CONTRACTOR TO ADDRESS ANY NEW CONDITIONS THAT MAY BE CREATED BY HIS ACTIVITIES AND TO PROVIDE ADDITIONAL FACILITIES, OVER AND ABOVE MINIMUM REQUIREMENTS, AS MAY BE NEEDED.
6. STOCKPILES ARE TO BE LOCATED IN SAFE AREAS AND ADEQUATELY PROTECTED BY TEMPORARY SEEDING AND MULCHING. HYDRO-SEED PREFERRED.
7. AREAS TO BE ROUGH GRADED WITH FINISH GRADING TO FOLLOW NEAR PROJECT COMPLETION ARE TO BE SEEDED WITH ANNUAL, PERENNIAL OR HYBRID RYE GRASS. THIS ALSO INCLUDES PERIMETER DIKES AND THE SEDIMENT BASIN EMBANKMENT. HYDRO-SEED PREFERRED.
8. IMMEDIATELY FOLLOWING FINISH GRADING, PERMANENT VEGETATION (CONSISTING OF RAPID, PERSISTENT AND LEGUME) WILL BE APPLIED. (MINIMUM 80# PER ACRE) THIS IS TO INCLUDE:
   20% ANNUAL, PERENNIAL OR HYBRID RYEGRASS
   40% CREEPING RED FESCUE, AND
   40% WHITE CLOVER
   HYDRO-SEED PREFERRED.
9. FERTILIZER: SHALL BE APPLIED AT 400# PER ACRE OF 10-20-20 (10 POUNDS PER 1100 SQUARE FEET) OR EQUIVALENT.
10. PREPARATION OF SURFACE: ALL AREAS TO BE SEEDED SHALL BE CULTIVATED TO THE SATISFACTION OF THE ENGINEER. THIS MAY BE ACCOMPLISHED BY DISCING, RAKING, HARROWING OR OTHER ACCEPTABLE MEANS.
11. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH 2 INCH STONE, AS CONDITIONS DEMAND, AND REPAIR AND/OR CLEANOUT OF ANY STRUCTURES USED TO TRAP SEDIMENT. ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
12. ALL FILLS AS DESIGNATED BUILDING SITES TO BE COMPACTED TO 90% OF MAX. DENSITY.

MAINTENANCE OF STRAW BALE

Straw bale barriers shall be inspected immediately after each rainfall and at least daily during prolonged rainfall.
Close attention shall be paid to the repair of damaged bales, end runs and undercutting beneath bales.
Necessary repairs to barriers or replacement of bales shall be accomplished promptly.
Sediment deposits should be removed after each rainfall. They must be removed when the level of deposition reaches approximately one-half the height of the barrier.
Any sediment deposits remaining in place after the straw bale barrier is no longer required shall be dressed to conform to the existing grade, prepared and seeded.

STANDPIPE AND SEDIMENT POND MAINTENANCE

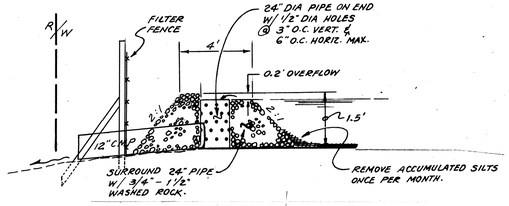
The embankment of the basin should be checked regularly to insure that it is structurally sound and has not been damage by erosion or construction equipment.
The emergency spillway should be checked regularly to insure that its lining is well established and erosion-resistant.
The siltation basin should be checked after each runoff-producing rainfall for sediment cleanout. When the sediment reaches the cleanout level, it shall be removed and properly disposed of.

GRADING QUANTITIES

CUT : 18,614 ± C.Y.

FILL : 14,344 ± C.Y.

APPROVED FOR ROAD CONSTRUCTION ONLY BY: G.E. WEED, DIRECTOR DEPT. OF PUBLIC WORKS DATE: 4/23/87



FILTER BERM STANDPIPE DETAIL

N.T.S.

NOTE :

ALL GRADING TO BE IN CONFORMANCE WITH U.B.C., CHAPTER 70.

RICHARD E. SCHMID — STATE OF WASHINGTON — 13357 — 10-22-86 — Registered PROFESSIONAL ENGINEER

REV. 2 12-11-86 RES
REV. 3 3-10-87 RES

REV. 5 : ADD ROCKERIES & ROCKERY DETAIL. 7-07-87 AJ
REV. 4 : REV GRADES @ INTER. 25TH AVE & 163RD ST. & ADD FILTER BERM-STDPIPE 4-10-87 AJ.
REV. 3 : ADD FILTER FENCE LOTS 4-8 3-6-87 AJ.
REV. 2 : REVISE CLEARING LIMITS AND ADDED GRADING PLAN. 12-11-86 DS.
REV. 1 : INCREASE CLEARING LIMITS TO ACCOMODATE THE NECESSARY BUILDING AREAS 10/23/86 ALT

TEMPORARY EROSION - SEDIMENTATION CONTROL PLAN CLEARING LIMITS GRADING PLAN

FOR

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

NO.	REVISION	DATE	BY

Western Surveyors Inc.
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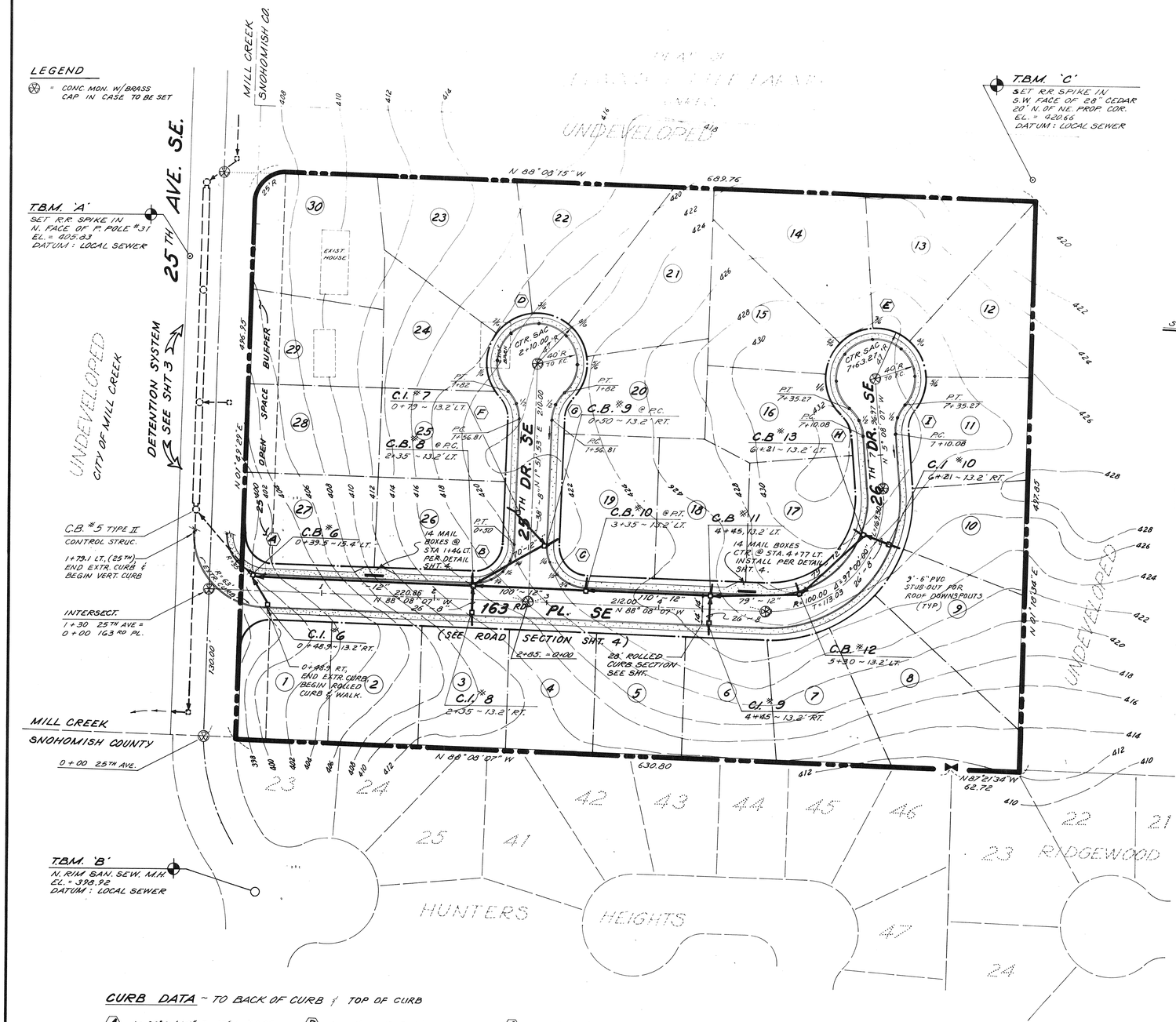
DWN. BY	DATE	PROJECT MANAGER	SCALE
KAM	8-26-86	EN. BONE	
CHKD. BY	DATE	SHEET	JOB NO.
EN.B.		5 OF 5	84-005

FIELD BOOK : 115

84-005

HDEV - 288

SW 1/4, SEC. 5, TWP. 27, R. 5 E., W.M.



SCALE : 1" = 50'

GENERAL NOTES

1. ALL WORK AND MATERIALS TO BE IN ACCORDANCE WITH SNOHOMISH COUNTY STANDARDS AND SPECIFICATIONS.
2. PRIOR TO ANY SITE WORK PERTAINING TO DRAINAGE, THE CONTRACTOR SHALL CONTACT THE CHIEF INSPECTOR FOR LAND DEVELOPMENT DIVISION AT (206) 259-9454 TO SET UP SCHEDULING. DUE TO FIELD CHANGES (REVISIONS), ENGINEERED AS-BUILTS WILL BE REQUIRED PRIOR TO SITE APPROVAL.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE VERIFICATION OF THE LOCATION AND DEPTH OF ALL EXISTING UTILITIES (WHETHER SHOWN ON THIS PLAN OR NOT) PRIOR TO BEGINNING OF CONSTRUCTION, SO AS TO AVOID DAMAGE OR DISTURBANCE. THE CONTRACTOR SHALL NOTIFY THE ENGINEER AT EARLIEST OPPORTUNITY REGARDING APPARENT UTILITY CONFLICTS.
4. ALL FILLS SHALL BE COMPACTED TO MINIMUM OF 95% OF MAXIMUM DENSITY. (DETERMINED BY THE MODIFIED PROCTOR TEST.)
5. GRADING OUTSIDE OF THE COUNTY RIGHT-OF-WAY MUST COMPLY WITH CHAPTER 70 OF THE UNIFORM BUILDING CODE.
6. STORM SEWER PIPE MAY BE:
   A. PLAIN CONCRETE PER AASHTO DESIGNATION M 86, CLASS 2. (RUBBER GASKET)
   B. CORRUGATED STEEL PIPE, 16 GAUGE, GALVANIZED W/ASPHALT TREATMENT #1 OR BETTER. (RUBBER GASKET)
   C. CORRUGATED ALUMINUM PIPE, 16 GAUGE IN R/W. (RUBBER GASKET)
   D. ALUMINIZED STEEL MEETING AASHTO DESIGNATION M 36, 16 GAUGE. (RUBBER GASKET)
   E. POLYVINYL CHLORIDE (P.V.C.) DRAIN PIPE METTING AASHTO DESIGNATION M 278, 8" DIAMETER MAXIMUM. P.V.C. NOT ALLOWED IN THE R/W. (RUBBER GASKET)

   ALL PIPE SHALL HAVE CLASS "B" BEDDING UNLESS OTHERWISE SHOWN.
7. DETENTION PIPE SHALL BE 14 GAUGE CORRUGATED STEEL PIPE 35"X73" CMP14 WITH 3"x1" CORRUGATIONS AND ASPHALT TREATMENT NO. 1. CONNECTING BANDS SHALL BE MINIMUM 18" WIDE, WITH RUBBER GASKETS. COMPACTION TESTS WILL BE REQUIRED AROUND THE PIPE. NATIVE MATERIAL WILL NOT BE ALLOWED FOR BACKFILL AROUND PIPE. 100% IMPORTED GRAVEL BASE CLASS B WILL ONLY BE ACCEPTABLE. CONTRACTOR WILL SUPPLY METHOD OF COMPACTION AND LIST EQUIPMENT PRIOR TO WORK STARTING. COMPACTION TESTS BY MODIFIED PROCTOR METHOD.
   ~~ALTERNATE PIPE MAY BE CORRUGATED ALUMINUM ALLOY, 14 GAUGE~~

GENERAL NOTES (CONTINUED)

All work and materials shall be in accordance with Snohomish County Standards and Specifications and Washington State Department of Transportation 1984 Standard Specifications for road, bridge, and municipal construction.

All work within the site and county right-of-way shall be subject to the inspection of the county engineer or his designated representative.

The temporary erosion/sedimentation control facility shall be constructed prior to any grading or extensive land clearing in accordance with the approved temporary erosion/sedimentation control plan. These facilities must be satisfactorily maintained until all construction and landscaping is completed and the potential for on-site erosion has passed.

Unless otherwise noted all storm sewer pipe shall be concrete (CP) nonreinforced, ASTM C-14 (24" diameter and larger to be reinforced, ASTM C-76), or corrugated metal (CMP): CMP to be galvanized steel with treatment 1 asphalt coating or better, or corrugated aluminum pipe, or AASHTO M274-70 aluminized steel. All pipes shall have rubber gaskets.

CP indicates concrete pipe will be required. CMP indicates corrugated metal pipe may be used instead of concrete pipe.

Areas to be rough-graded with finish grading to follow near project completion are to be seeded with annual, perennial or hybrid rye grass. This includes perimeter dikes and the sediment basin embankment. HYDRO-SEED preferred.

Immediately following finish grading, permanent vegetation (consisting of rapid, persistent and legume) will be applied. (Minimum 80# per acre) Thise is to include the following:

20% Annual, perennial or hybrid ryegrass
40% Creeping Red Fescue
40% White Clover

HYDRO-SEED preferred.

FERTILIZER: Shall be applied at 400# per acre of 10-20-20 (10 pounds per 1100 square feet) or equivalent.

PREPARATION OF SURFACE: All areas to be seeded shall be cultivated to the satisfaction of the county inspector. This may be accomplished by discing, raking, harrowing or other acceptable means.

PIPE SPECIFICATIONS:
Galvanlize steel CMP shall meet the requirements of AASHTO designation M-36, type 1 & type 2. Pipe to be helical with 2 2/3" X 1/2" corrugations and shall have asphalt treatment 1 or better.

STEEL

Gage	Pipe	Arch	Band Size
16	12"-36"	17x13 thru 42x29	12"
14	42"-48"	49x33	18"
12	54"-60"	57x38 & 64x43	24"

ALUMINUM

Gage	Pipe	Arch	Band Size
16	12"-27"	17x13 thru 42x29	12"
14	30"-36"	28x20 & 35x24	18"
12	42"-54"	42x29 & 49x33	18"
10	60"	57x38 & 64x43	24"

All non-perforated metal pipe shall have neoprene gaskets at the joints.

Backfill trench of new utilities shall be compact to 95% relative compaction under roadwy and 90% relative compaction off roadways, as specified in Section 2-03.3(14)D and Section 2-03.3(14)B.

Prior to sidewalk construction, the County Inspector will evaluate individual lot drainage and direct the installation of catch basin stubouts and or behind sidewalk french drains as required. Stubs shall be marked with a 2"x 4". Locations of these installations shall be placed on the as-built construction plans and submitted to the County.

Storm water retention/detention facilities must be constructed and maintained in accordance with Snohomish County requirements.

Provide and maintain the temporary sedimentation collection facilities to insure sediment laden waters does not enter the natural drainage system.

All disturbed area such as retention facilities, roadway back-slopes, ect. shall be seeded with a perennial ground cover grass to minimize erosion. Grass seeding will be done using an approved HYDROSEEDER or as otherwise approved by Snohomish County.

All earth work shall be performed in accordance with county standards. Pre-construction soils investigation may be required to evaluate soils stability.

If cut and fill slopes exceed a maximum of two feet horizontal to one foot vertical, a rock or concrete retaining wall will be required. All rock retaining walls are to follow county specifications and to be designed and certified by a civil engineer experienced in soils mechanics.

Vegetation shall be established on areas disturbed or on areas of construction as necessary to minimize erosion.

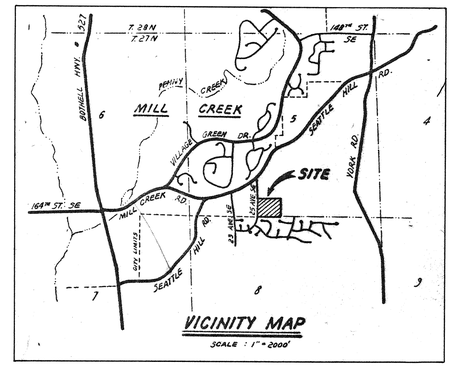
Stockpiles are to be located in safe areas and adequately protected by temporary seeding and mulching. HYDRO-SEED preferred.

GENERAL NOTES (CONTINUED)

8. A SURVEY OF THE DETENTION SYSTEM IS REQUIRED TO CERTIFY THE ORIFICE SIZE, F.W.O.P., DIMENSIONS, AND DETENTION CAPACITY OF THE SYSTEM "AS BUILT". CERTIFICATION BY A CIVIL ENGINEER IS REQUIRED.
9. ALL PIPE SHALL BE PLACED ON STABLE EARTH, OR IF IN THE OPINION OF THE COUNTY ENGINEER, THE EXISTING FOUNDATION IS UNSATISFACTORY, THEN IT SHALL BE EXCAVATED BELOW GRADE AND BACK-FILLED TO GRADE WITH SAND-GRAVEL, CRUSHED STONE OR OTHER SUITABLE MATERIAL. NEVER INSTALL PIPE ON SOD, FROZEN EARTH OR LARGE BOULDERS OR ROCK.
10. THE BACKFILL SHALL BE PLACED EQUALLY ON BOTH SIDES OF THE PIPE OR PIPE-ARCH IN LAYERS WITH A LOOSE AVERAGE DEPTH OF 6", MAXIMUM DEPTH 8", THOROUGHLY TAMPING EACH LAYER. SINGLE COMPACTED LAYERS MUST EXTEND FOR ONE DIAMETER OR EACH SIDE OF THE PIPE OR TO THE SIDE OF THE TRENCH. MATERIALS TO COMPLETE THE FILL OVER PIPE SHALL BE THE SAME AS DESCRIBED. (REFER TO WSDOT STD. SPEC. 7-04.3(3) AND STD. SPEC. 2-03.3(14)C, METHOD B & C).
11. ALL DISTURBED SLOPES SHALL BE SODDED OR SEEDED WITH A PERENNIAL GROUND COVER (GRASS) TO PREVENT EROSION.
12. ALL GRATES (INLET) SHALL BE DEPRESSED 0.1 FEET BELOW PAVEMENT LEVEL.
13. ALL CATCH BASINS TO BE TYPE I UNLESS OTHERWISE NOTED.
14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING ALL MANHOLE, INLET, AND CATCH BASIN FRAMES AND GRATES JUST PRIOR TO POURING OF CURBS AND PAVING.
15. ALL CATCH BASINS WITH A DEPTH OVER 5.0 FEET TO THE FLOWLINE SHALL BE TYPE II CB'S (MANHOLE).
16. ALL TYPE II CATCH BASINS SHALL HAVE LOCKING LIDS.
17. STANDARD LADDER STEPS SHALL BE PROVIDED IN ALL CATCH BASINS/MANHOLES EXCEEDING 4 FEET IN DEPTH.
18. CATCH BASIN FRAME AND GRATES SHALL BE OLYMPIC FOUNDRY MODEL 5435, 5435A OR 50503A, LOCKING TYPE OR EQUAL. MODEL 5435A IS REFERRED TO AS A "THROUGH INLET" ON THE PLAN. MODEL 50503A IS REFERRED TO AS A "ROLLED GRATE INLET" ON THE PLAN.
19. BACKFILL TRENCH OF CULVERT PIPES IN LIFTS OF 6" - 8" AND COMPACT TO 95% RELATIVE COMPACTION UNDER ROADWAYS AND 90% RELATIVE COMPACTION OFF ROADWAYS.
20. ALL ROOF TOP DRAINS SHALL BE DIRECTED INTO THE STORM DRAINAGE SYSTEM VIA THE NEAREST CATCH BASIN UNLESS OTHERWISE INDICATED.
21. THE CONTRACTOR SHALL MAKE MINOR ADJUSTMENTS IN THE FIELD DURING CONSTRUCTION IF NECESSARY IN ORDER TO PRODUCE A FINISHED PROJECT CONSISTENT WITH THE PLAN. THE CONTRACTORS SHALL KEEP 2 COPIES OF THE PLANS AVAILABLE ON WHICH TO RECORD AS-BUILT DATA. AT JOB COMPLETION ONE SET SHALL BE PROVIDED TO THE ENGINEER SO A PERMANENT AS-BUILT RECORD MAY BE DEVELOPED.

INDEX

SHT. NO.		DESCRIPTION
1	~	PLAT, ROAD & STORM PLAN
2	~	PLAT, ROAD & STORM PROFILES
3	~	PLAN & PROFILE (25TH AVE. SE)
4	~	DETAILS
5	~	TEMP. EROSION - SEDIMENTATION CONTROL PLAN



CURB DATA ~ TO BACK OF CURB & TOP OF CURB

(A) Δ = 90°07'24" R = 35' L = 50.05
P.C. = 0+48.9 = 400.00 (163 RD)
1/4 Δ = 400.37
1/2 Δ = 400.74
3/4 Δ = 401.10
P.T. = 1+78.9 = 401.47 (25TH AVE.)

(B) Δ = 90°00'00" R = 36' L = 56.50
P.C. = 2+35 = 412.13 (163 RD)
1/4 Δ = 413.03
1/2 Δ = 413.94
3/4 Δ = 414.84
P.T. = 0+30 = 415.74 (25TH DR)

(C) Δ = 90°00'00" R = 36' L = 56.50'
P.C. = 0+30 = 415.74 (25TH DR.)
1/4 = 416.38
1/2 = 417.01
3/4 = 417.65
P.T. = 3+35 = 418.28 (163 RD)

(D) Δ = 26°49'50" R = 40' L = 187.68
1/8 Δ = 44°48'18"
P.C. = P.T. 1+82 = 418.20
1/8 Δ = 418.51
2/8 Δ = 418.90
3/8 Δ = 419.70
4/8 Δ = 419.39
5/8 Δ = 419.00
P.T. = P.C. = 1+85 = 418.20

(E) Δ = 26°49'50" R = 40' L = 187.68
1/8 Δ = 44°48'18"
P.C. = P.T. = 7+35.27 = 426.69
1/8 Δ = 427.00
2/8 Δ = 427.45
3/8 Δ = 428.18
4/8 Δ = 427.45
5/8 Δ = 427.00
P.T. = P.C. = 7+35.27 = 426.69

(F) Δ = 44°24'55" R = 36' L = 27.91'
P.C. = 1+56.81 = 417.80
1/2 Δ = 418.00
P.T. = 1+82 = 418.20

(G) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 1+56.81 = 417.80
1/2 Δ = 418.00
P.T. = 1+82 = 418.20

(H) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 7+10.08 = 426.35
1/2 Δ = 426.52
P.T. = 7+35.27 = 426.69

(I) Δ = 44°24'55" R = 36' L = 27.91
P.C. = 7+10.08 = 426.35
1/2 Δ = 426.52
P.T. = 7+35.27 = 426.69

CONSTRUCTION SCHEDULE

1) INSTALL T.E.S.C.P. (SEE SHT. 5)
2) CLEARING & GRADING (SEE SHT. 5)
3) SEWER AND WATER MAIN (A.W.D. PLANS)
4) STORM DRAINAGE FACILITIES
5) CURBS AND SIDEWALKS
6) PAVE STREETS

RICHARD E. SCHMIDT 14376 REGISTERED PROFESSIONAL ENGINEER 10-12-86

REV. 1 12-11-86 RES
REV. 2 3-6-87 RES

APPROVED FOR CONSTRUCTION: Randolph R. Sleight for G.E. WEED, DIRECTOR DATE: 4/23/87
SNO. CO. DEPT. OF PUBLIC WORKS

MAILBOX LOCATIONS
APPROVED BY: MICHAEL MAHER DATE: FEB. 27, 1987
SUPERINTENDENT POSTAL OPERATIONS, BOTHELL

NO.	REVISION	DATE	BY
REV. 4	INCREASE C.D.S.'s R/W'S & FACE OF CURB RADII TO 47'R & 40'R RESPECTIVELY	4-21-87	AJ
REV. 3	REV. STORM SYSTEM E2M'S & 10.3 RD	4-10-87	A.J.
REV. 2	ADD REV. GEN. NOTES, WALKS AROUND C.D.S.'s, REV. CURB DATA, ADD CONST. SCHED., RE # C.B.'s, & CI.'s, AND ADD C.I. #9 & C.B. #11	3-05-87	A.J.
REV. 1	ADDED STUB OUT FOR ROOF DRAINS, DELETED C.I. 9 & DR. ESM'T BETW. LOT 1 & 2.	12-11-86	D.S.

ROAD & STORM PLAN

FOR:

MILL CREEK SOUTH

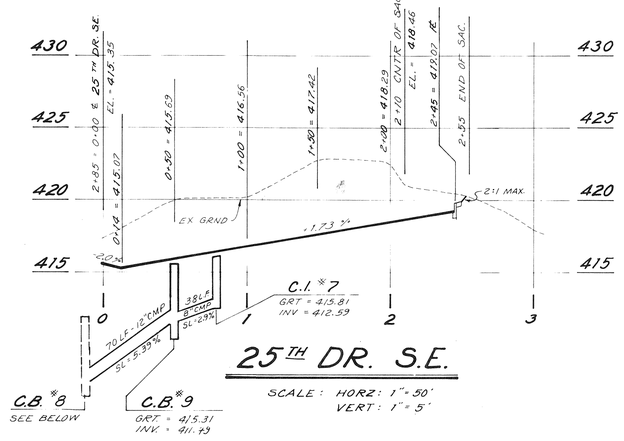
DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

Western Surveyors Inc.
745-1594 or 355-2776
LAND USE CONSULTANTS
CIVIL ENGINEERS • LAND SURVEYORS
13322 Highway 99 South • Everett, WA 98204 • (206) 745-1594

DWN BY: KAM	DATE: 8-26-86	PROJECT MANAGER: E.V. BONE	SCALE:
CHKD BY: E.V.B.	DATE:	SHEET: 1 OF 5	JOB NO.: 84-005

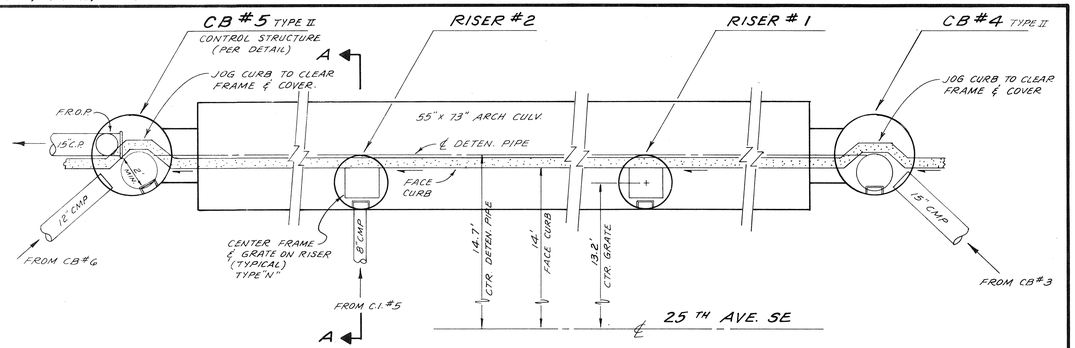
HDE1-422

MILL CREEK SOUTH FIELD BOOK: 115 84-005



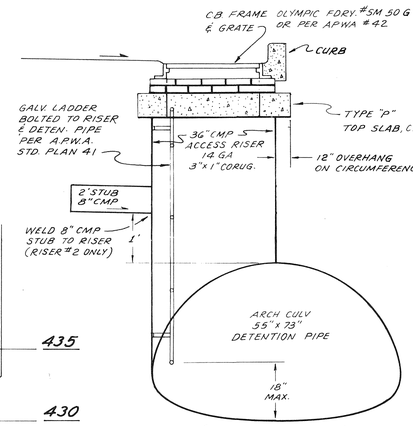
25TH DR. S.E.

SCALE: HORZ: 1" = 50'
VERT: 1" = 5'



DETENTION SYSTEM PLAN VIEW

NO SCALE

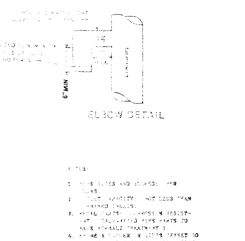


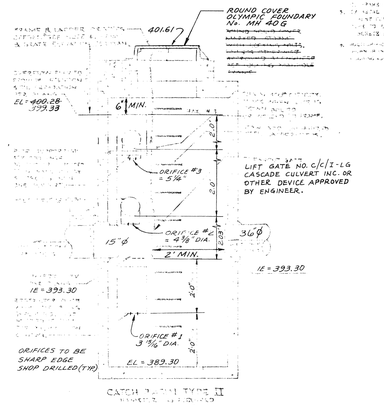
(SECTION A-A)

ACCESS RISER DETAIL

NO SCALE

NOTE:
COMPACT HAUNCH AREA TO 95% MAX. DENSITY. CLASS 8' BEDDING AND BACKFILL TO GRADE

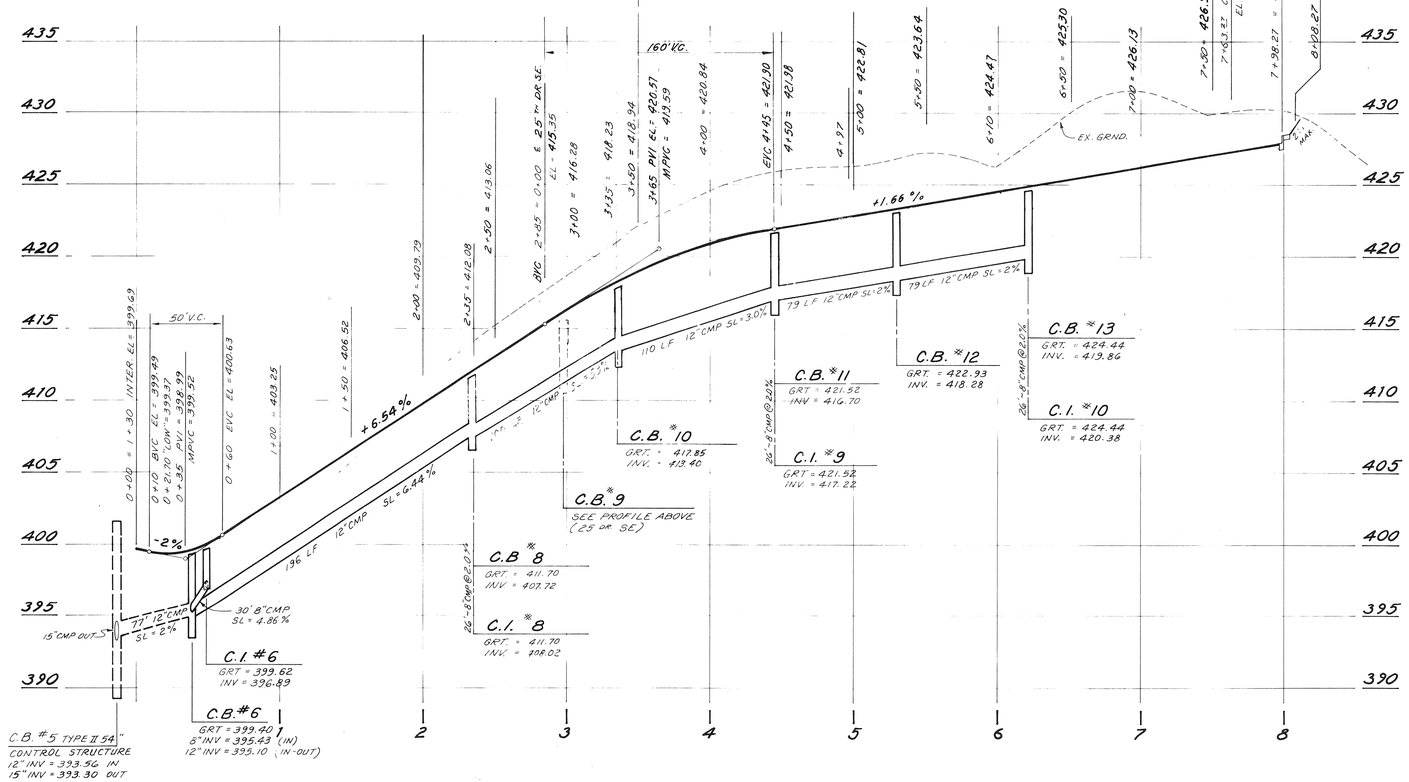




MULT-ORIFICE F.R.O.P. CONTROL DEVICE

C.B. #4 DETAIL

NO SCALE



163RD PL. S.E. & 26TH DR. S.E.

SCALE: HORZ: 1" = 50'
VERT: 1" = 5'

APPROVED FOR CONSTRUCTION Randolph W. Slaughter BY: G.E. WEED, DIRECTOR DEPT. OF PUBLIC WORKS DATE: 4/23/87

REV. 2: REV. ROAD & STORM PROFILES @ INTERSECTION 25TH AVE & 163 RD PL 4-10-87 A.J.
REV. 1: REV. ROAD & STORM DR. PROFILE. 12-11-86 D.S.

REV. 1, 12-11-86 RES
REV. 2 3-6-87 RES

ROAD & STORM PROFILES

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NO	REVISION	DATE	BY

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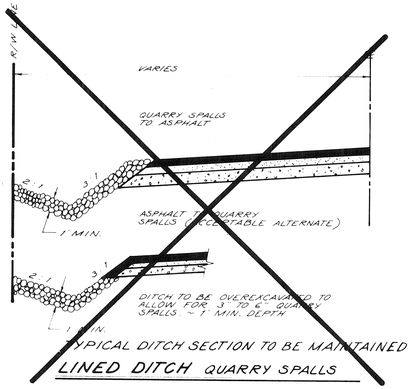
DWN BY	DATE	PROJECT MANAGER	SCALE
KAM	9-29-86	E.V. BONE	AS SHOWN
CHKD BY	DATE	SHEET	JOB NO.
EVB		2 OF 5	84-005

MILL CREEK SOUTH HDEV-423 JOB NO. 84-005

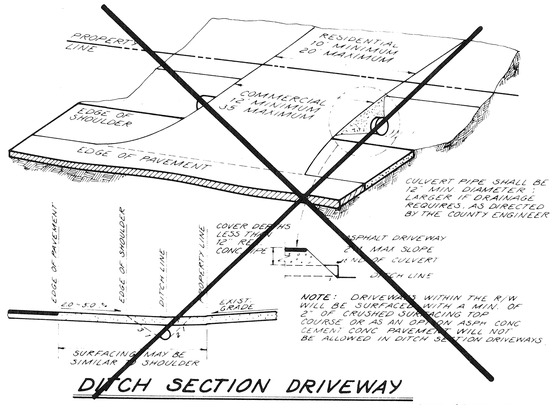
S.W. 1/4, SEC. 5, TWP. 27, R.5 E, W.M.

MILL CREEK SOUTH HDEV-424 JOB NO. 84-005

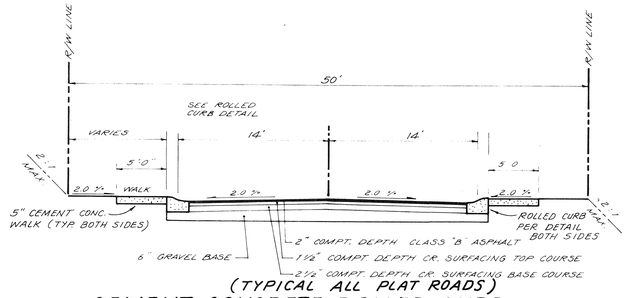


TYPICAL DITCH SECTION TO BE MAINTAINED

LINED DITCH QUARRY SPALLS

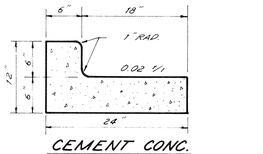


DITCH SECTION DRIVEWAY

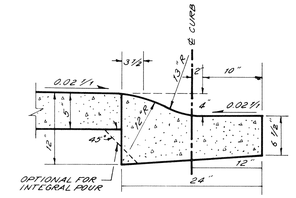


(TYPICAL ALL PLAT ROADS)

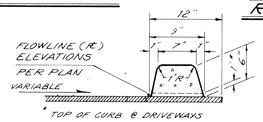
CEMENT CONCRETE ROLLED CURB
CURB ROADWAY SECTION



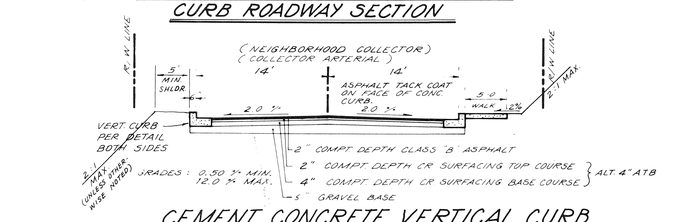
CEMENT CONC. VERTICAL CURB & GUTTER



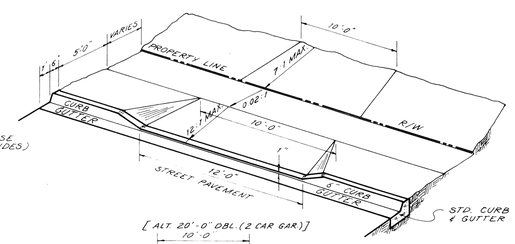
CEMENT CONC. ROLLED CURB

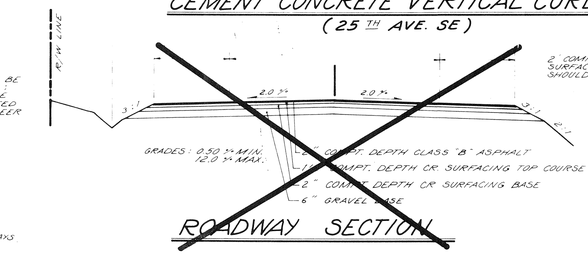


EXTRUDED ASPHALT OR CEMENT CONC. CURB



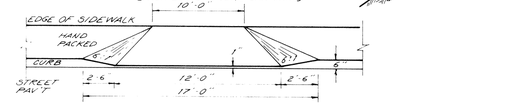
CEMENT CONCRETE VERTICAL CURB
(25TH AVE. SE)





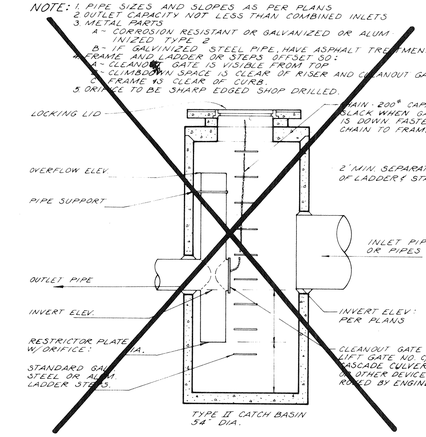
ROADWAY SECTION

2" COMPT. DEPTH CR. SURFACING TOP COURSE SHOULDERS (BOTH SIDES)

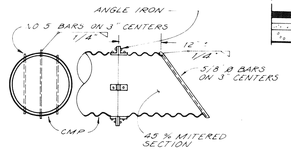


CEMENT CONC. OR ASPHALT CONC. DRIVEWAY DETAILS

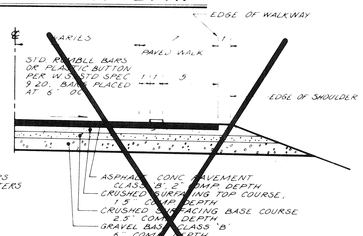
NOTE: 1. PIPE SIZES AND SLOPES AS PER PLANS
2. OUTLET CAPACITY NOT LESS THAN COMBINED INLETS
3. METAL PARTS
A - CORROSION RESISTANT OR GALVANIZED OR ALUMINIZED TYPE 2
B - IF GALVANIZED STEEL PIPE HAVE ASPHALT TREATMENT
4. FRAME AND LADDER OR STEPS OFFSET 30°
A - CLEANOUT GATE IS VISIBLE FROM TOP
B - CLEAR OPEN SPACE IS CLEAR OF RISER AND CLEANOUT GATE
C - FRAME IS CLEAR OF CURB
5. GRATE TO BE SHARP EDGED SHOP DRILLED



F.R.O.P. CONTROL DEVICE DETAIL



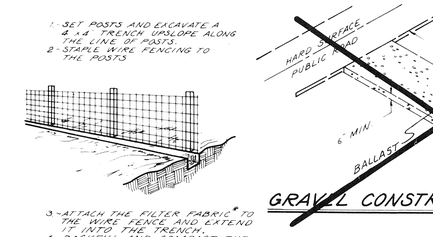
TRASH SCREEN DETAIL

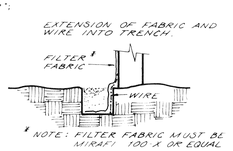


ROADWAY SECTION

THE RUMBLE BARS SHALL BE A SOLID POLYESTER CASTING OF THE FOLLOWING DIMENSIONS: WIDTH - 3", LENGTH - 12", AND HEIGHT - TAPERING FROM 1" AT THE EDGE OF THE BAR TO 1/4" ALONG THE LONGITUDINAL CENTERLINE. THE LEADS OF THE BAR SHALL BE BEVELED FROM THE TOP OF THE 1/4" EDGE AT A 1:1 SLOPE.

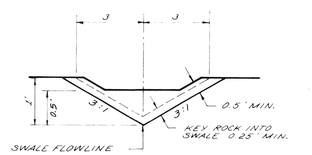
STANDARD WALKWAY



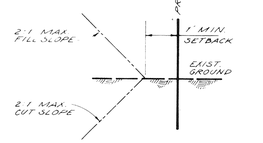


FILTER FENCE DETAIL

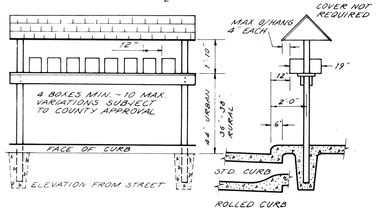
GRAVEL CONSTRUCTION ENTRANCE



INTERCEPTOR DITCH X-SECTION & ROCK CHECK DAM



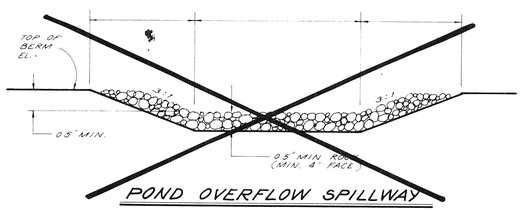
CUT & FILL REQUIREMENTS



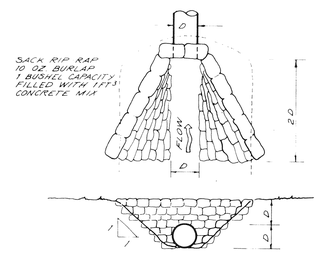
MAILBOX STANDARD

GENERAL NOTES:
1. - MAILBOX MUST BE TYPE "APPROVED BY THE POST-MASTER GENERAL" WITH A UNIFORM BOX STYLE AND METHOD OF ADDRESS IDENTIFICATION PER EACH STANDARD.
2. - LOCATION IS SUBJECT TO APPROVAL BY THE COUNTY FOR PROTECTION OF VIEWS AND ACCESS.
3. - THE SKETCH DEPICTS A MINIMUM STRUCTURAL AND DIMENSIONAL STANDARD. INNOVATIVE DESIGNS MEETING THE MINIMUM DIMENSIONAL AND STRUCTURAL REQUIREMENTS ARE ACCEPTABLE.
4. - POST MUST GIVE FIRM SUPPORT BUT NOT TO EXCEED 1 7/8" DIA. IRON PIPE OR 4"x4" WOODEN POST.

APPROVED FOR CONSTRUCTION — G.E. WEED, DIRECTOR — DATE: 4/23/87
DEPT. OF PUBLIC WORKS



POND OVERFLOW SPILLWAY

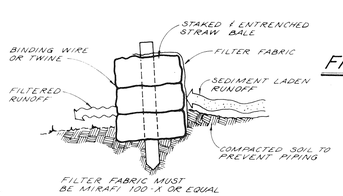


SACK RIP RAP 10 OZ BURLAP 1 BUSHEL CAPACITY, FILLED WITH 1:1:3 CONCRETE MIX

DO NOT MITER PIPE END

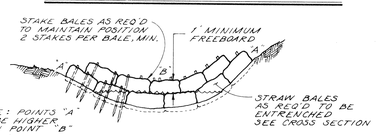
ROCK 8" DIA. OR GREATER MAY BE SUBSTITUTED FOR SACK IF MORTARED TO HOLD IN PLACE

HEADWALL
SACK RIP-RAP DETAIL



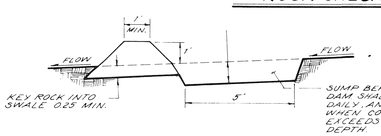
FILTER FABRIC MUST BE MIRAFI 100 X OR EQUAL

STRAW BALE CROSS-SECTION



NOTE: POINTS "A" TO BE HIGHER THAN POINT "B"

STRAW BALE DETAIL



NOTE: ROCK SHALL BE 4" MINUS QUARRY ROCK

ROCK CHECK DAM SECTION

TYPICAL INTERCEPTOR DITCH WITH ROCK CHECK DAM

R.E. Schmidt 10-22-86

Lou Schmidt 4-16-87

REV. 1: REV. RD. SEC (25TH) 3/6/87 A.J.

NO.	REVISION	DATE	BY

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DWN BY	DATE	PROJECT MANAGER	SCALE
K.A.M.	8-26-86	E.J. BONE	AS SHOWN
CHKD BY	DATE	SHEET	JOB NO
RES		4 OF 5	84-005

REV. 1 3-10-87

DETAILS

FOR:

MILL CREEK SOUTH

DEVELOPER:
KEN LONG & DICK SCHMIDT
13322 HWY. 99 SOUTH
EVERETT, WA 98204
PHONE (206) 745-1594

PLAT OF
ELWOOD LITTLE FARMS
UNREC.

25TH AVE. S.E.

EXIST. HOUSE

CLEARING LIMITS

CLEARING LIMITS

PROPOSED CONTOUR (TYP.)

ROCKERY PER DETAIL

SPACE 100' BUFFER

25' OPEN

FILTER FENCE
(PER DETAIL SHT 4)
INSTALL 1 EAST OF R/W LINE.

FILTER BERM W/ STANDPIPE
(PER DETAIL)

EXISTING EARTH BERM
TOP = 393.3 ±

CLEARING LIMITS

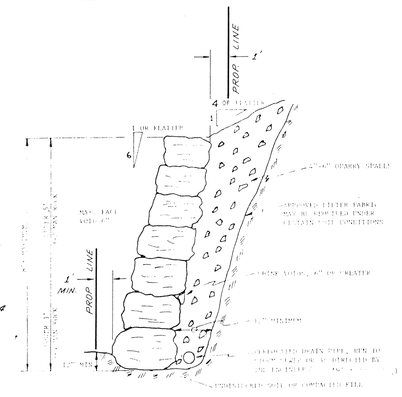
INSTALL ROCKERY PER DETAIL AS NEEDED TO MATCH EXISTING GROUND SLOPES.

INSTALL FILTER FABRIC FENCE OR STRAW BALES PER DETAIL SHT. 4. ALONG LOTS 4 THRU 8

HUNTERS HEIGHTS

RIDGEWOOD

SCALE : 1" = 50'



THE LONG DIMENSION OF THE ROCKS SHALL EXTEND INTO THE EARTH TO PROVIDE MAXIMUM STABILITY.

THE ROCK SHALL BE PLACED SO AS TO LOCK EACH ROCK INTO ITS FINAL POSITION.

2 MAN ROCKS 300 LBS. MIN., 800 LBS. MAX., SIZE 14" TO 18"
3 MAN ROCKS 800 LBS. " 1500 LBS. MAX., SIZE 18" TO 22"
4 MAN ROCKS 1500 LBS. " 2100 LBS. MAX., SIZE 22" TO 24"
5 MAN ROCKS 2100 LBS. " 3000 LBS. MAX., SIZE 30" TO 38"

ANY ROCKERY GREATER THAN 8 FEET IN HEIGHT MUST BE DESIGNED AND INSPECTED BY A LICENSED STRUCTURAL ENGINEER. THE ENGINEER MUST CERTIFY TO THE CITY THAT THE ROCKERY WAS INSTALLED IN ACCORDANCE WITH PLANS AND SPECIFICATIONS.

ROCKERIES 7 FEET AND LOWER SHALL BE CONSTRUCTED OF A MAX. OF 2 MAN ROCKS FROM BOTTOM TO TOP.
ROCKERIES 7 FEET AND HIGHER SHALL BE CONSTRUCTED OF A MAX. OF 2 MAN ROCKS FROM BOTTOM TO TOP.

ROCKERY DETAIL

NO SCALE

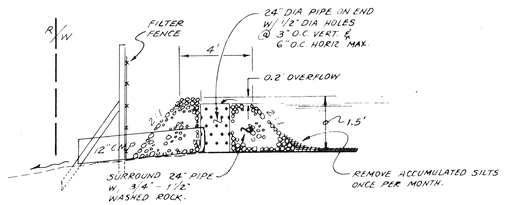
12. ALL FILLS AS DESIGNATED BUILDING SITES TO BE COMPACTED TO 90% OF MAX. DENSITY.

MAINTENANCE OF STRAW BALE

Straw bale barriers shall be inspected immediately after each rainfall and at least daily during prolonged rainfall.
Close attention shall be paid to the repair of damaged bales, end runs and undercutting beneath bales.
Necessary repairs to barriers or replacement of bales shall be accomplished promptly.
Sediment deposits should be removed after each rainfall. They must be removed when the level of deposition reaches approximately one-half the height of the barrier.
Any sediment deposits remaining in place after the straw bale barrier is no longer required shall be dressed to conform to the existing grade, prepared and seeded.

STANDPIPE AND SEDIMENT POND MAINTENANCE

The embankment of the basin should be checked regularly to insure that it is structurally sound and has not been damage by erosion or construction equipment.
The emergency spillway should be checked regularly to insure that its lining is well established and erosion-resistant.
The silitation basin should be checked after each runoff-producing rainfall for sediment cleanout. When the sediment reaches the cleanout level, it shall be removed and properly disposed of.



FILTER BERM STANDPIPE DETAIL

N.T.S.

NOTE:
ALL GRADING TO BE IN CONFORMANCE WITH U.B.C., CHAPTER 70.

R.E. Schmidt 10-22-86

REV. 2 12-11-86 RES
REV. 3 3-10-87 RES

GRADING QUANTITIES

CUT : 18,614 ± C.Y.

FILL : 14,344 ± C.Y.

APPROVED FOR Randolph R. Slight BY: G.E. WEED, DIRECTOR DATE : 4/23/87
FOR CONSTRUCTION DEPT. OF PUBLIC WORKS

REV. 5 : ADD ROCKERIES & ROCKERY DETAIL. 7-07-87 AJ
REV. 4 : REV GRADES @ INTER. 25TH AVE & 163RD ST. & ADD FILTER BERM - STDPIPE 6-10-87 AJ.
REV. 3 : ADD FILTER FENCE LOTS 4-8 3-6-87 AJ.
REV. 2 : REVISE CLEARING LIMITS AND ADDED GRADING PLAN. 12-11-86 DS.
REV. 1 : INCREASE CLEARING LIMITS TO ACCOMODATE THE NECESSARY BUILDING AREAS 10/29/86 AJ

TEMPORARY EROSION - SEDIMENTATION CONTROL PLAN
CLEARING LIMITS
GRADING PLAN

FOR

MILL CREEK SOUTH

DEVELOPER:
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MILL CREEK SOUTH FIELD BOOK : 115 HDEV-426 84-005